

FINAL REPORT

Project Title: S31: In-line Timber Bridge Replacement Options
(2017/18)

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SUMMARY

Queensland Department of Transport and Main Roads (TMR) is currently managing over 300 ageing timber bridges, of which the residual service lives are potentially being consumed by the increasing freight demand. Some of these bridges may require replacing in near future to meet the road demand and ensure safe operation.

Replacement of timber bridges typically occurs in-line. Traditional replacement bridge construction requires long bridge closure and a side track to facilitate construction. The cost of construction of the side track has been a concern due to the high cost often approaching the cost of replacement of the structure itself. Rapid construction methods have been identified as a viable option to avoid side track construction while minimising road closure time. Therefore, this report reviews the current market options for rapid construction of replacement bridges, which will allow replacement of timber bridges in a relatively short time.

Several pre-engineered bridge products which are currently available on Australian market have been identified and reviewed for their compliance with TMR bridge design criteria and specifications. In addition, this report proposes a three-step selection process with graphical initial screening, a matrix-based preliminary selection and detailed review for final selection.

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1 INTRODUCTION

1.1 Background

Queensland Department of Transport and Main Roads (TMR) is responsible for the management of over 300 timber bridges. Most of these ageing structures are designed to design standards that are insufficient in comparison to today's standards and level of service needs. Many of these structures constructed between the 1920's and 1960's have been in service for over 60 years and are currently approaching the high maintenance phase of their life. It is expected that the freight demands are likely to consume the residual service life of these assets at an increasing rate.

Currently, replacement of timber bridge structures typically occurs in the same alignment (in-line) as the existing bridge. This requires the construction of a temporary side track to facilitate construction, with the cost of the side track often approaching the cost of replacement of the structure itself.

This project aims to review proven rapid construction techniques available domestically. Key project criteria include the ability to employ a cost-effective and durable solution in a relatively short construction time with minimal disruption to traffic whilst maintaining the current alignment and road geometry.

1.2 Project Objectives

The objectives of this project are as follows:

1. Characterise the bridge, road and route configurations where TMR would typically target timber bridge in-line replacement, specifically determining available road closure windows and pre-works constraints. This characterisation may also be suitable for bridges that would not be typically done in-line, but that is not the focus of the characterisation.
2. Review and collate feasibility level cost based on recent projects to determine a baseline cost for typical projects.
3. Review and collate proven rapid bridge construction solutions, specifically collating their alignment with (1).
4. Evaluate bridge systems and identify any departures and non-compliance to *Design Criteria for Bridges and Other Structures* (TMR 2018) and relevant technical specifications.
5. Develop a framework 'Timber Bridge Replacement Options Selection Matrix'.
6. Populate (5) with the data developed in (2) to (4) and provide a supporting report.
7. Prepare a supporting database with more details on available options.

1.3 Scope

The scope of the project is to:

1. determine typical functional requirements for in-line timber bridge replacement projects
2. determine baseline cost for each system as nominated by the supplier (list in the option selection matrix)
3. determine available market solutions that align with (1)
4. provide a matrix to rapidly inform decision-makers of feasible options for in-line replacement based on typical project and product characterisation.

Exclusions to scope are:

- solutions that are bespoke (i.e. not substantially pre-engineered)
- culvert solutions
- non-timber bridge in-line replacement projects
- market solutions that do not comply with in-line replacement functional requirements
- solutions that are not currently available in the Australian market and not designed to Australian standards (e.g. AS 5100:2017 Series).

1.4 Limitations

- This study does not undertake a detailed engineering review of the bridge systems for TMR acceptance for construction.
- This report tabulates parameters of available proprietary bridge systems at the time of writing. The objective of the bridge system selection methodology outlined in this report is only to shortlist the bridge systems that are meeting project specific requirements. It does not imply that TMR has approved the use of these bridge systems.
- TMR acceptance of a new bridge system shall follow the TMR Engineering Innovation process. Inquiries regarding this Innovation process shall be sent to:

Department of Transport and Main Roads,
Director (Structures Design, Review and Standards),
GPO Box 1412,
Brisbane City. Qld 4000

2 TMR IN-LINE TIMBER BRIDGE REPLACEMENT CONSIDERATIONS

2.1 In-line Replacement Considerations

Bridges are a crucial element of a road network which require strategic planning and maintenance for safe operation. Closure or service restriction of a bridge can make significant social and economic impact, particularly in rural areas where there are no alternative routes. Most in-service timber bridges are significantly aged and have consumed most of their safe and useful life, creating many challenges in maintenance and safe operation and increasing the pressure on authorities to replace them.

Replacement of timber bridges has historically been based on several factors including asset criticality (how critical the asset's continued function is to stakeholders).

The remaining timber bridge assets in the TMR network are now usually on secondary routes. Typically, replacement occurs in-line; however, a number of site factors can significantly influence the suitability of in-line bridge replacement options, such as:

1. traffic volume
2. availability of an accessible alternative route
3. stream bed stability and accessibility (can most of the stream bed be reliably accessed during at least part of the year to facilitate in-line construction?)
4. site geometry
5. lack of environmental constraints associated with activities in and around the water course
6. predictable weather patterns
7. the capacity of associated road networks to accommodate construction traffic and heavy prefabricated elements
8. the condition of the structure to be replaced (Utilising the existing timber structure as a work platform)
9. the feasibility of extensive pre-works (prior to road shut down) and availability of suitable laydown areas to facilitate construction activities including pre-works.
10. local availability of suitable construction plant (particularly cranes) and construction materials.

A very favourable site is likely to include a short total length structure (minimises craneage and logistics issues) with simple site geometry (straight road crossing straight stream at 90 degrees) and a readily available alternate traffic route (minimum impact on service disruption) with low traffic volumes. A readily accessible stream bed with an adjacent flat laydown area during part of the year (facilitates pre-works, craneage and multiple work fronts) with locally available large cranes and construction workforce is also very beneficial. However, it is unlikely for a site to have all these favourable conditions, which imposes constraints on replacement. For example, most of the remaining TMR timber bridges are in rural areas with limited resources and therefore, large machines and skilled workforce may not be locally available.

In addition to the site factors listed above, suitable in-line replacement solutions that complement the favourable site include:

1. a competitive cost base
2. compliance with TMR bridge design criteria and specifications
3. short lead time

4. maximum use of pre-engineered components (particularly superstructure)
5. flexible integration of (3) into a tailored solution (pre-engineered design can be adapted to a range of sites)
6. extensive pre-fabrication for reduction of in situ works
7. compatibility with pre-works opportunities associated with the site
8. robust elements that are not susceptible to damage during construction
9. minimum site time and road closure requirements
10. immediate trafficability following practical completion (i.e. no cure time required for construction materials)
11. lightweight elements (minimises heavy machinery requirements).

Again, some of these desirable attributes tend to be mutually exclusive, and regardless of the above, geotechnical conditions vary for each site. Hence, solutions will require flexibility in substructure configuration, and timely geotechnical data will be required to enable design and project completion.

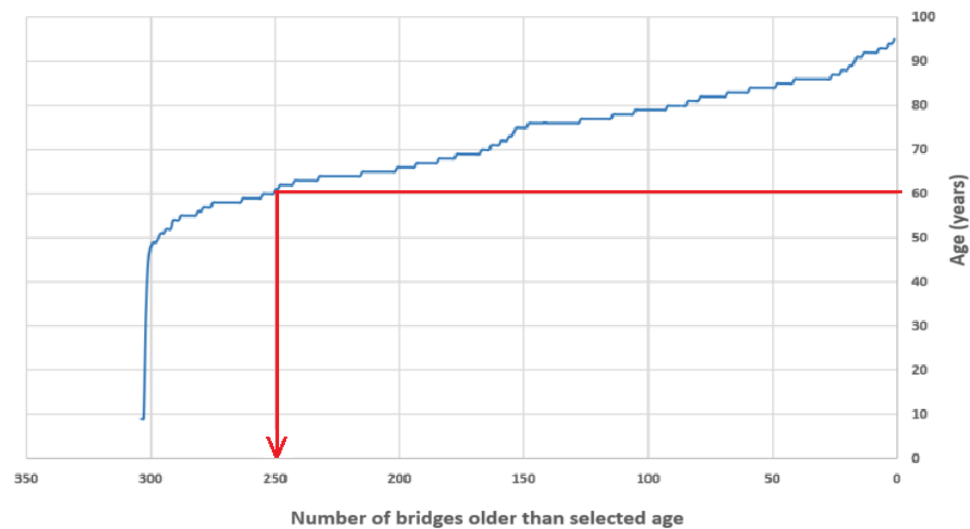
Combining a favourable site with a suitable in-line bridge replacement solution will involve compromises. Given that most sites are less than favourable, and in-line bridge replacement market solutions are often constrained by material, fabrication or construction requirements, it is essential that functional requirements and priorities for the site are clearly understood and documented. This investigation has developed tools to facilitate this process, namely:

1. site condition screening
2. replacement bridge system option selection matrix
3. bridge system data base.

2.2 TMR Bridge Stock Attributes

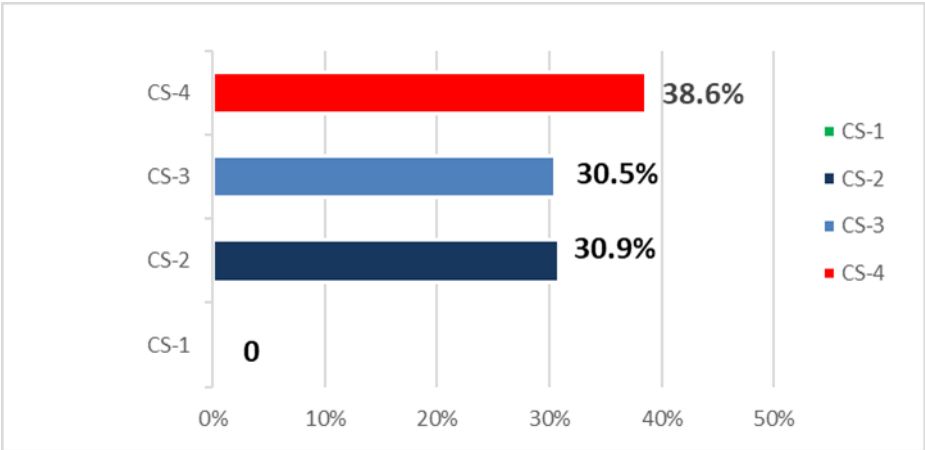
As per TMR bridge information system (BIS) data (as at October 2017), 304 timber bridges distributed over the Queensland remain in-service on TMR managed roads. As shown in Figure 2.1, approximately 250 (80%) of these bridges are more than 60 years old and were designed for a much lower traffic load than those built to current standards. Further, as shown in Figure 2.2, approximately 70% of the TMR timber bridge stock is currently in condition state 3 or 4 and approaching the high maintenance phase of their service life. Distribution of in-service timber bridges across six TMR regions and their condition rating is shown in Figure 2.3.

Figure 2.1: Age of TMR timber bridges

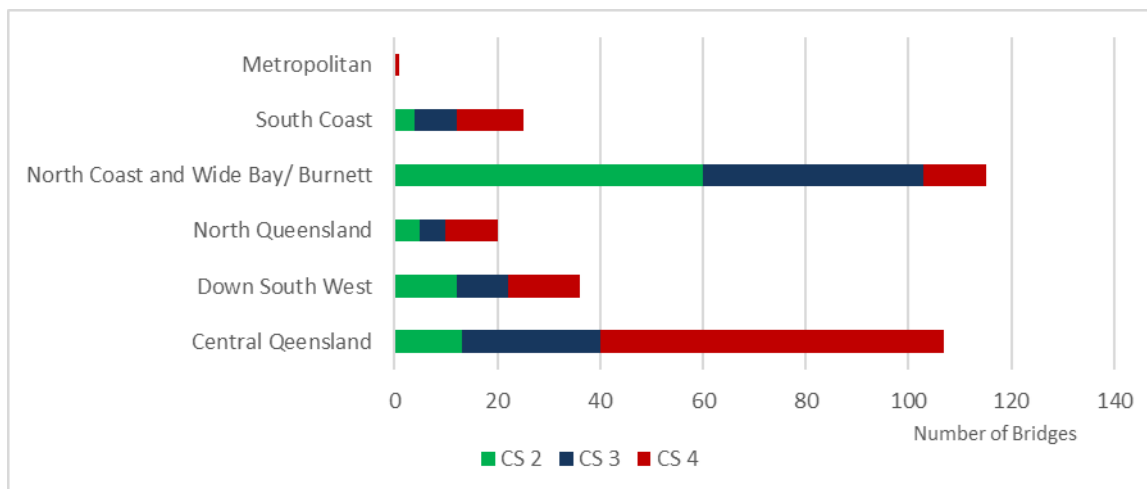


Source: TMR BIS (as per October 2017).

Figure 2.2: Overall condition rating of in-service timber bridge stock



Source: TMR BIS (as per October 2017).

Figure 2.3: Distribution and overall condition rating of timber bridges in TMR regions

Source: TMR BIS (as per October 2017).

Timber bridges are usually multi-span structures with over 90% of TMR timber bridges having individual span lengths of less than 10 m as shown in Table 2.1. Further, as shown in Figure 2.4 approximately 80% of timber bridges have an overall length of less than 40 m. Sixty per cent of the timber bridge stock accommodate two traffic lanes, and the remaining 40% of the stock have a single traffic lane only. According to bridge inventory data, carriageway width of single lane bridges varies from 2.1 m to 8.5 m with 65% of them measuring less than the current recommended minimum width of 4.9 m. Carriageway width of current two-lane timber bridges varies from 3.6 m to 9 m with 99% of them measuring less than the TMR's recommended minimum width of 8.6 m (TMR 2018). Almost all of these bridges have timber plank decks, but the substructure materials vary as shown in Figure 2.5.

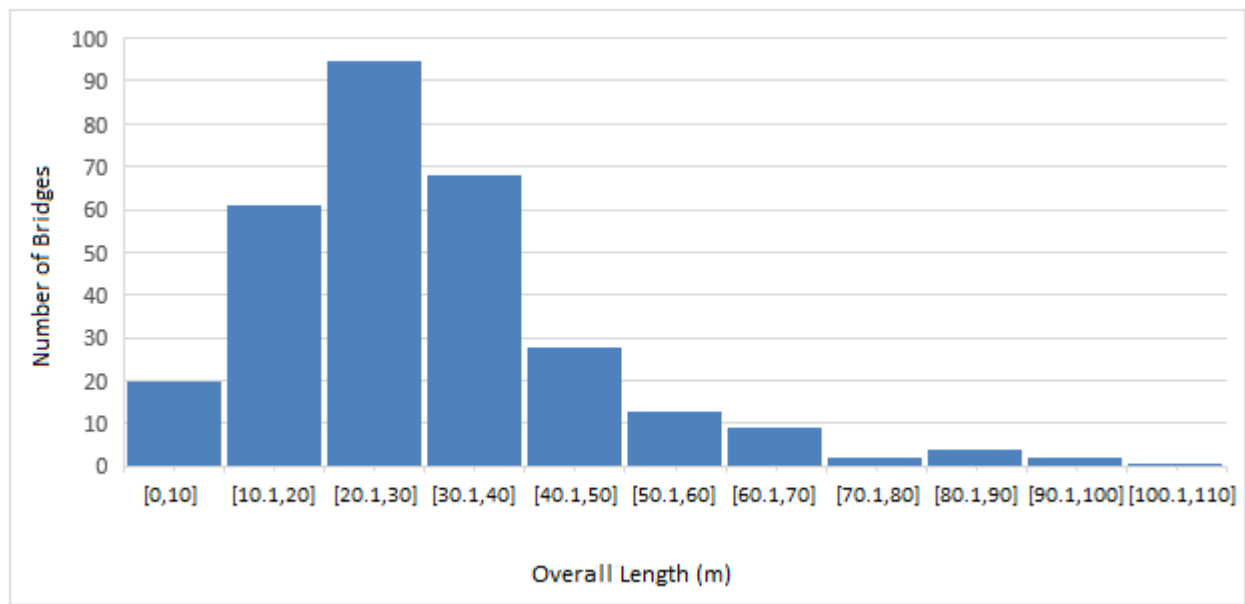
Table 2.1: Total timber bridge stock by average span length and number of spans

| | | Average span lengths | | | Total bridges |
|-----------------|----|----------------------|-------------|--------------|---------------|
| | | 0–5.0 m | 5.01–10.0 m | 10.01–15.0 m | |
| Number of spans | 1 | 1 | 17 | 5 | 23 |
| | 2 | 1 | 46 | 4 | 51 |
| | 3 | 2 | 88 | 7 | 97 |
| | 4 | 14 | 58 | | 72 |
| | 5 | | 21 | 4 | 25 |
| | 6 | | 14 | | 14 |
| | 7 | | 8 | 2 | 10 |
| | 8 | | 3 | 1 | 4 |
| | 9 | | 4 | | 4 |
| | 10 | | 1 | | 1 |
| | 11 | | 1 | | 1 |
| | 16 | | 1 | | 1 |
| Total bridges | | 18 | 262 | 23 | 303 |

Note: Colour code - 1 88

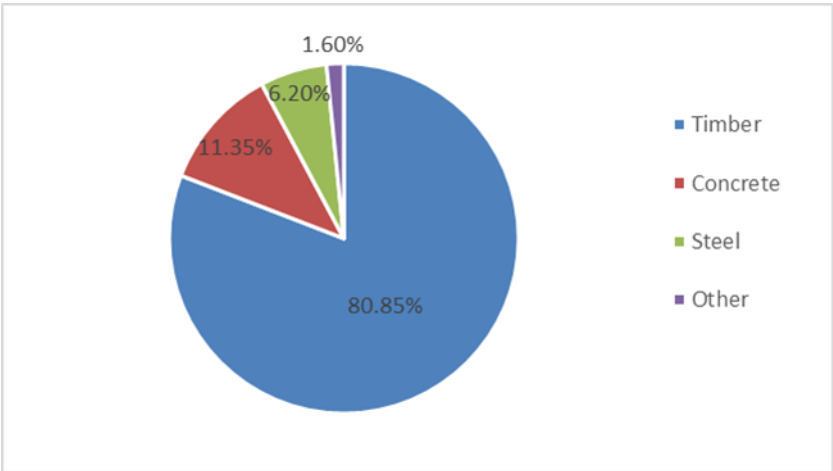
Source: TMR BIS (as per October 2017).

Figure 2.4: Overall length of TMR timber bridges



Source: TMR BIS (as per October 2017).

Figure 2.5: Substructure materials of TMR timber bridges



Source: TMR BIS (as per October 2017).

2.3 Compliance with TMR Bridge Design Criteria and Specifications

Section 2.1 discussed the constraints that may apply when conducting an in-line replacement and identified some characteristics of the ideal site. However, investigations and studies done on current market options for replacing bridges revealed possible deviations from the TMR bridge design criteria and specifications. As such, compromises may be necessary to select a replacement option, which will also consider factors such as the site condition, lead time etc. For any scenario where a departure from TMR documentation is necessary to allow a replacement option as outlined in this report, the impact of that departure must be assessed and appropriately managed/mitigated.

Commonly reported deviations in current market options which would be a departure from the *Design Criteria for Bridges and Other Structures* (DCBOS) (TMR 2018) and other relevant TMR specifications are listed below in Table 2.2.

Table 2.2: Common deviations in current market options from TMR bridge design criteria and specifications

| Criteria/component | Common deviations | TMR recommendations | Reference clause |
|----------------------------|---|---|--|
| Driven piles | Reinforced concrete piles, spun piles, steel piles | Reinforced concrete piles, spun piles, Steel piles are not allowed | Clause 4.1.6 – DCBOS Clause 4.3 – DCBOS |
| Bored piles | Oversized pre-bored pile holes | Maximum oversize of 55 mm to a depth of 3 m | Section 6 – MRTS65 |
| Design exposure conditions | Design for B1 exposure | Minimum B2 exposure | Clause 10.4.5 – DCBOS |
| Bridge barrier | Use of 'W' or 'Thrie' beams Low performance barriers | 'W' or 'Thrie' beams are not allowed Minimum of regular performance barriers | Clause 4.9.1.1 – DCBOS Clause 4.9.5.1 – DCBOS |
| Bearing | Not provided or no provision for replacing | Required as per DCBOS | Clause 4.6.1.2 – DCBOS Clause 4.6.2 – DCBOS |
| Relieving slab | No provision or non-compliant detailing | Provide as per standard drawing 2255 | Clause 4.11 – DCBOS Standard Drawing 2255 |
| Narrow bridges | Narrower than minimum carriageway width | Minimum carriageway width of 4.9 m for single lane and 8.6 m for 2 lanes | Clause 3.1 – DCBOS |
| Expansion joints | Open joints | Open joints are not permitted | Clause 4.10.2 – DCBOS |

Note:

- DCBOS – Design Criteria for Bridges and Other Structures (TMR 2018).
- MRTS- Transport and Main Roads Technical Specification (Current as of time of writing)

3 PRE-ENGINEERED SOLUTIONS

The traditional method of replacing bridges requires full bridge closure for the duration of demolition, site preparation and construction. Traffic is usually diverted through alternative roads or a temporary side track. Many timber bridges are positioned in regional roads with no suitable alternative routes. Diverting traffic through other routes may significantly increase the travel distance and time. Additionally, construction of a side track alternate route may be prohibitively expensive.

Pre-engineered bridge solutions offer the opportunity to minimise closure time at sites and, in certain circumstances, potentially remove the need for side track establishment. ARRB reviewed currently available pre-engineered solutions to evaluate their applicability to TMR's timber bridge network. The following pre-engineered solutions were reviewed:

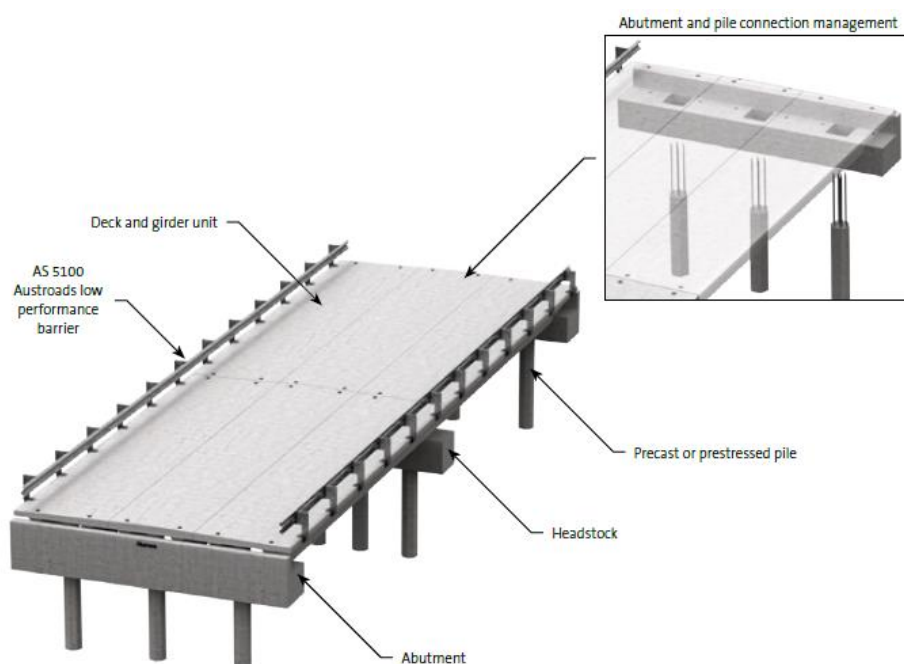
- HumeDeck Modular Bridge System (Section 3.1)
- InQuik Bridging System (Section 3.2)
- M-Lock Precast Bridge (Section 3.3)
- UniBridge (Section 3.4)
- Country Bridge Solution (Section 3.5)
- Compact 200- Mabey (Section 3.6).

Note: Refer to Section 1.4 of this report for TMR acceptance process for these bridge systems.

3.1 HumeDeck Modular Bridge System

The HumeDeck modular bridge system is a precast bridge with prestressed concrete superstructure and precast reinforced concrete substructure that has been developed by Holcim Australia. It is a complete bridge system with an assembly as shown in Figure 3.1 and structural form as shown in Table 3.1. Refer to Appendix B.1 for further details.

Figure 3.1: Complete HumeDeck system assembly



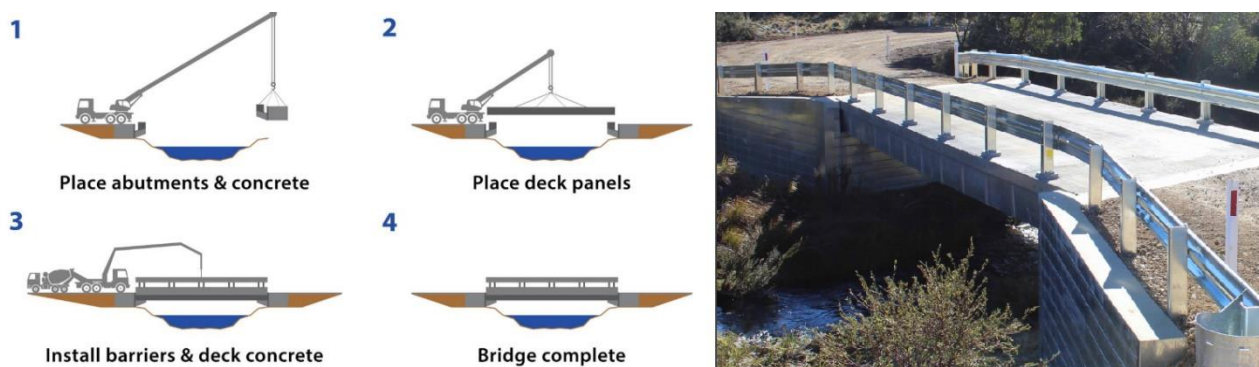
Source: Holcim Australia (2011).

Table 3.1: Structural form of HumeDeck modular bridge

| | |
|------------|--|
| Deck | Deck panels with integral girders and grouted joints (no topping concrete) |
| Abutments | Custom-designed precast concrete unit |
| Pier | Headstock on piles |
| Headstock | Custom-made precast concrete unit bolted to piles/existing piers |
| Foundation | Precast piles (potted or driven) or use the existing foundation |
| Wing walls | Custom-designed precast units |

3.2 InQuik Bridging System

The InQuik bridging system is a fully reinforced in situ cast concrete structure with permanent sacrificial steel formwork which allows rapid construction as shown in Figure 3.2. The bridge typically comes as a fully integrated structure (simply supported decks are also available) with the structural form shown in Table 3.2. The lightweight steel formwork with prefabricated reinforcement allows quick and easy erection of the bridge deck within a few hours. The deck concrete is usually poured the following day. Refer to Appendix B.2 for further details.

Figure 3.2: InQuik bridging system

Source: InQuik Pty Ltd (2017).

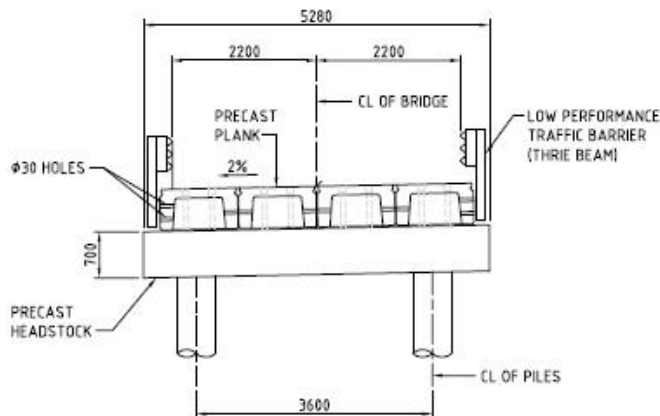
Table 3.2: Structural form of InQuik bridging system

| | |
|------------|--|
| Deck | Fully reinforced in situ cast concrete structure with permanent sacrificial steel formwork (protects concrete) |
| Abutments | Fully reinforced in situ cast concrete abutments with permanent sacrificial steel formwork |
| Pier | Fully reinforced concrete pier with the left in place sacrificial steel formwork |
| Headstock | Fully reinforced in situ cast concrete headstock with permanent sacrificial steel formwork |
| Foundation | Steel H-Piles, strip footing, poured/driven concrete piles, screw piles |
| Wing walls | Cantilevered wing walls off the main abutment – fully reinforced concrete structure |

3.3 M-Lock Precast Bridge

M-Lock is a complete precast concrete bridge system designed by Rocla Pty Ltd, which has been used in over 250 bridges in Australia. The M-Lock bridge uses inverted U sections for the deck and reinforced concrete precast items for other bridge components in the form shown in Table 3.3. Figure 3.3 shows a typical bridge section and a completed bridge.

Figure 3.3: M-Lock precast bridge



Source: Rocla Pty Ltd (2012).

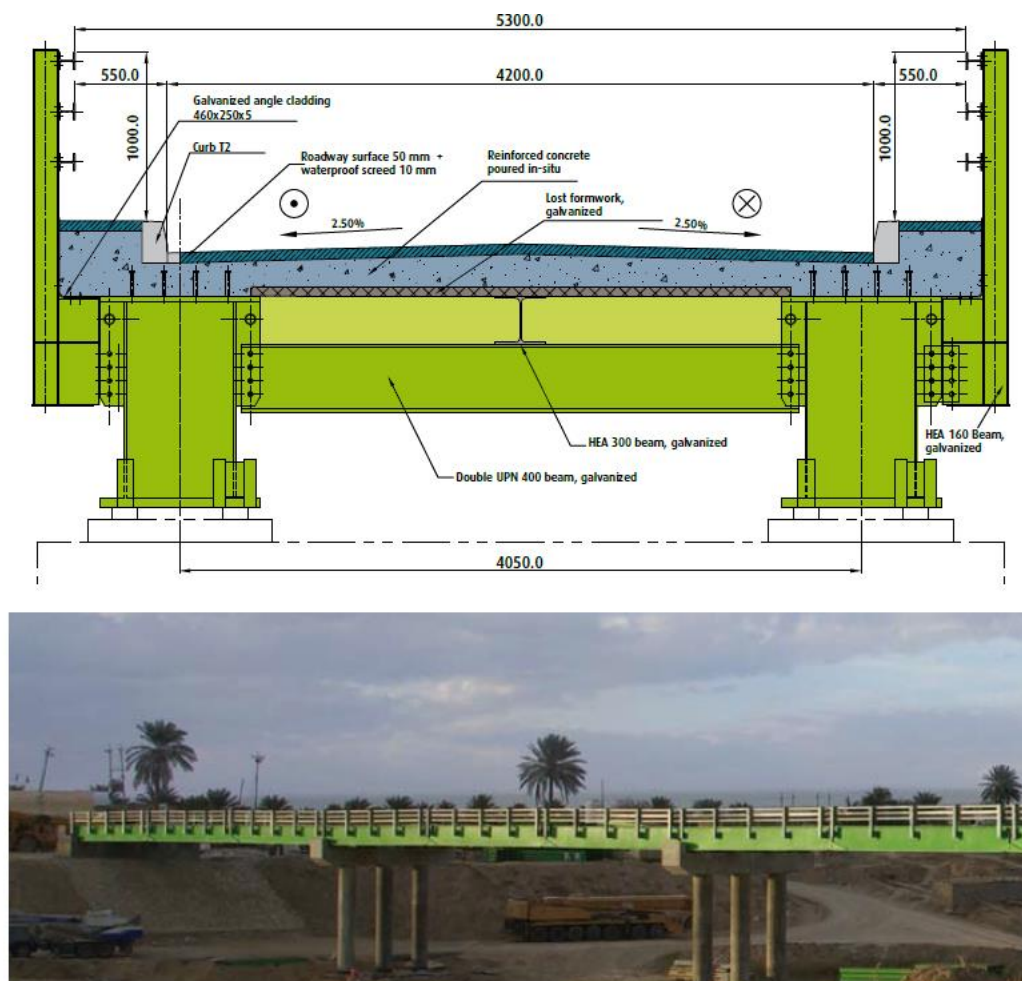
Table 3.3: Structural form of M-Lock precast bridge

| | |
|------------|---|
| Deck | Precast concrete inverted U sections fitted to headstock using bolts (no topping slab) |
| Abutments | Rocla MassBloc |
| Pier | Precast headstock on piles |
| Headstock | Precast concrete unit |
| Foundation | Socketed precast piles – hollow precast piles, square RC piles, steel UC or tubular piles |
| Wing walls | Rocla MassBloc |

3.4 UniBridge

UniBridge is a steel pre-engineered bridge developed and supplied by UniBridge Australasia Pty Ltd. This system consists of a complete superstructure only in the form of a prefabricated steel deck or in situ poured concrete deck with steel formwork. The superstructure consists of two prefabricated steel box girders and bolted cross girders. It is compatible with all types of substructures and typical designs for substructures are available from UniBridge, if required. Figure 3.4 shows a typical cross-section of a bridge with composite deck and a completed bridge structure.

Figure 3.4: Typical bridge section with a composite deck and a completed UniBridge

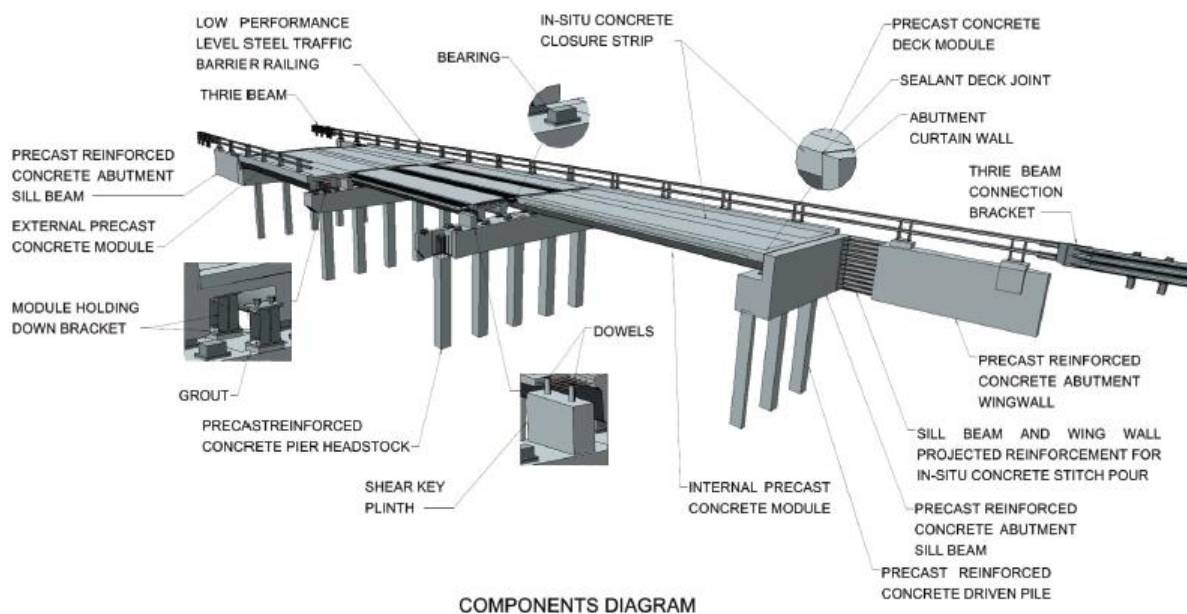


Source: UniBridge Trading (2018).

3.5 Country Bridge Solution (CBS)

Country bridge solution is a pre-engineered precast bridge system designed by Roads and Maritime Services (RMS). It consists of a precast prestressed concrete deck of inverted double T deck units with in situ closure pour and a precast concrete headstock. RMS provides the design only with a complete set of fully certified (to AS 5100-2004) drawings for the superstructure and headstock. The contractor has to design the structure below the headstock for site conditions. Typical designs are available to adopt. CBS is available for anyone to produce; however, at the time of writing, there is no current manufacturer. Figure 3.5 shows the components of CBS and Table 3.4 shows the structural form of different components of CBS.

Figure 3.5: Components of CBS



Source: RMS (2017).

Table 3.4: Structural form of CBS

| | |
|------------|---|
| Deck | Precast prestressed concrete double-T deck units and in situ closure pour |
| Abutments | Precast sill beam on pile/column supports |
| Pier | Pile/column on spread footing depending on site conditions |
| Headstock | Precast concrete unit on pile/column supports |
| Foundation | Spread footing/precast driven piles |
| Wing walls | Precast concrete units |

3.6 Compact 200- Mabey

Compact 200 (C 200) as shown in Figure 3.6 is a truss/bailey bridge developed by Mabey Bridge Ltd for temporary and permanent applications. The bridge can be rapidly erected with minimum machinery and using unskilled labour. C 200 can span long lengths up to 60.96 m with no intermediate supports. Mabey supplies the superstructure only. However, it is compatible with any type of substructure and can be launched with incremental launching.

Figure 3.6: C-200 bridge superstructure

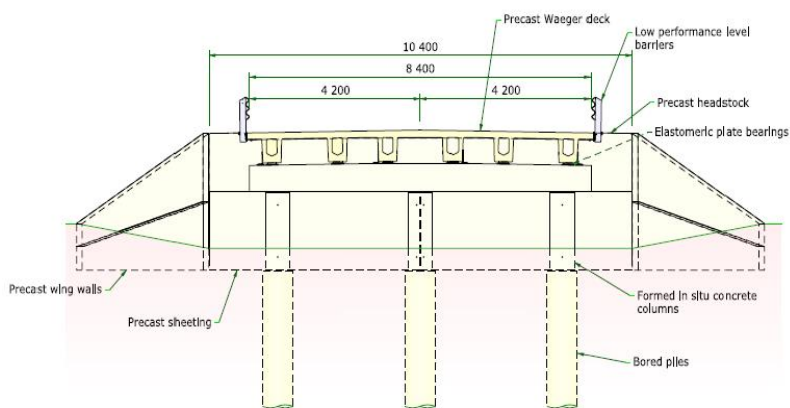


Source: Mabey Bridge Ltd (2018).

3.7 Waeger Precast Bridge

Waeger precast bridge is a complete bridge system which is designed, supplied and constructed by Waeger Constructions Pty Ltd. It is generally custom-designed to best fit each site and then precast to minimise in situ concrete work and rapid construction. Figure 3.7 shows a typical cross-section, deck units and a completed Waeger bridge. Table 3.5 gives the structural form of bridge components.

Figure 3.7: Waeger precast bridges



Source: Documents supplied by Waeger Construction Pty Ltd.

Table 3.5: Structural form of Waeger bridge

| | |
|------------|---|
| Deck | Precast deck units |
| Abutments | Precast abutments |
| Pier | Cast in situ columns |
| Headstock | Precast unit |
| Foundation | Bored cast in piles, driven concrete piles or steel piles |
| Wing walls | Precast panels with bolted connections |

3.8 Other Venders

Other than the pre-engineered bridges listed above, the vendors listed below also offer pre-engineered/rapid-built bridges. These vendors were contacted and offered an opportunity to provide further information but did not respond. Without further information, the compliance of these products with TMR bridge design criteria and specifications is unknown.

The following is a list of these vendors and their pre-engineered products:

- Hollow Core Concrete Pty Ltd – Hollow core concrete plank bridge
- Wagners – Composite fibre bridges
- Mabey Australia –Has a few pre-engineered bridge products. Except for C 200 listed in Section 3.6, other products are currently not in stock in Australia but can be ordered from UK. Mabey Australia does not wish to release product information about these bridges.
- Contech Engineered Solutions – Has few patented rapid-built precast concrete bridging systems. Their products are currently not available in Australia, but they are willing to introduce them. However, they are not currently designed to Australian standards.

4 OTHER SOLUTIONS

4.1 General Considerations

Bridge asset replacement/upgrade options generally include:

1. demolishing the bridge with no replacement (usually associated with reduced level of service requirements)
2. rehabilitating the existing bridge
3. in-line replacement including rapid construction
4. developing a conventional bespoke bridge replacement solution.

The 'do nothing' option, which is normally a consideration from an asset management perspective, is not feasible unless it is associated with a very short time limit. In general, the traditional bespoke solution (4) is the best option for replacing any structure. However, this process is normally associated with long construction times, high cost, limited availability of local materials and labour, and possibly road realignment or construction of a side track.

4.2 Rapid Construction Methods

This section summarises a range of methods to facilitate rapid construction including:

1. pre-engineered bridges
2. utilisation of the existing foundations
3. utilisation of the same superstructure for temporary and permanent bridge
4. realigning the road
5. pre-assembled bridges.

Generally, these concepts are not mutually exclusive.

4.2.1 *Pre-engineered Bridges*

Currently available pre-engineered bridge systems in Australia have been discussed in Section 3. The use of pre-engineered bridge systems has some advantages over bespoke solutions, typically constructability and speed of construction, as discussed in Section 2.1. Due to the nature of the pre-engineered bridges, deviation from standard solutions may result (see Section 2.3).

4.2.2 *Utilisation of the Existing Foundation*

As shown in Figure 2.5, 11.35% of current timber bridges have concrete substructures. Several modular deck replacement options which are mountable on any type of substructure are available to replace the superstructure. This can significantly reduce the construction time and cost but may require compromise to the design requirements depending on the substructure condition and capacity.

4.2.3 *Utilisation of the Same Superstructure for Temporary and Permanent Bridge*

Steel superstructures discussed in Section 3 such as Mabey bridge (C200) and UniBridge can be utilized as the temporary bridge for the side track during the construction of a replacement bridge. Once the substructure is completed, the assembled superstructure can be lifted and repositioned on the permanent structure in a matter of hours. Furthermore, these steel superstructures can span over 50 m in some instances, which may be beneficial at certain sites.

4.2.4 *Realigning the Road*

For some bridges, minor realignment of the road profile may be possible where the available road reserve is accommodating. In such cases, it has been a common practice to build the replacing permanent structure on the new alignment and divert the traffic at the end of construction. This can minimise the disturbance to traffic during the construction time and reduce the cost of a temporary side track while allowing the ability to deliver a better final product. Figure 4.1 shows a similar construction with CBS to replace an old timber bridge in NSW.

Figure 4.1: Bridge replacement on a new alignment



Source: RMS (2016).

4.2.5 *Pre-assembled Superstructure Installation*

The use of pre-assembled or prefabricated bespoke superstructures is another proven rapid construction/replacement method which uses a fully prefabricated/pre-assembled steel or composite bridge deck. This method reduces the amount of onsite work. However, large cranes may be required for lifting and placing the assembled structure in to position.

Figure 4.2: Pre-assembled superstructures



Source: BG&E (2018) (left) and Federal Highway Administration (2017) (right).

5 DECISION SUPPORTING TOOL

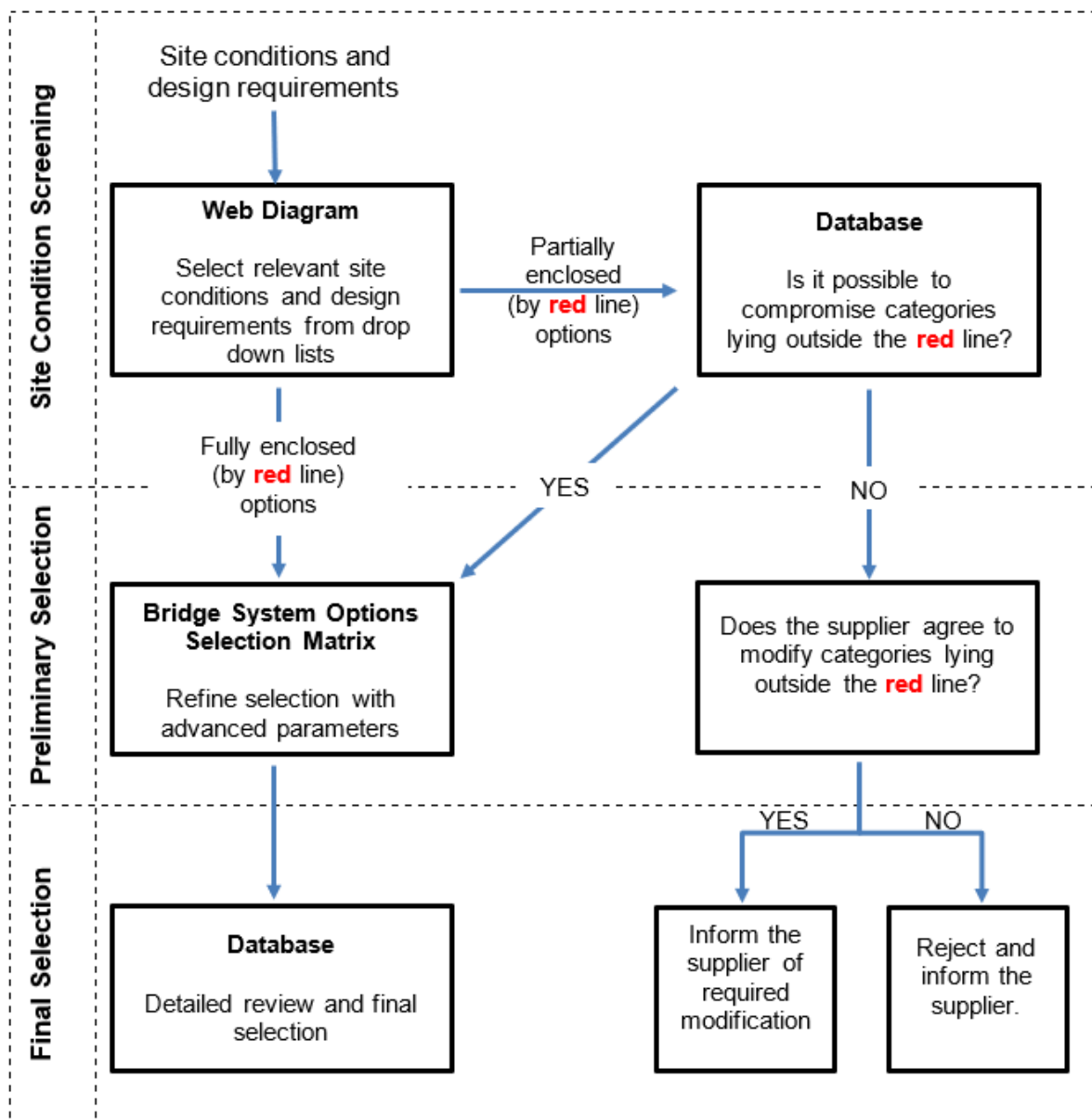
5.1 Selection Process

Outcomes of this project will facilitate TMR in the selection of available options to replace (in-line) timber bridges. The selection process proposed in this report consists of three steps as outlined below:

1. Site conditions screening – Section 5.2
2. Preliminary selection – Section 5.3
3. Final selection – Section 5.4.

The selection process should follow the sequence shown in Figure 5.1 with further explanation included in Section 5.2 – Section 5.4.

Figure 5.1: Bridge replacement option selection process – Flow chart



5.2 Site Conditions Screening

In order to simplify the selection process, an initial graphical screening has been developed in the form of a web diagram. The purpose of this step is to screen the replacement options based on a few main parameters as shown in Figure 5.2.

Figure 5.2: Site condition screening

Site Condition Screening

| Category | Parameter | Site condition/ design requirement (Please select the best from the list/ Enter) |
|-----------------------|---|---|
| Site Accessibility | Maximum allowable GVM (tonne) | |
| | Long vehicle access | - Choose from the list - |
| | Accessibility for pre-works | - Choose from the list - |
| Bridge Componentry | Superstructure | - Choose from the list - |
| | Substructure | - Choose from the list - |
| | Foundation | - Choose from the list - |
| Geometry | Span length (m) | |
| | Number of traffic lanes | - Choose from the list - |
| | Foot path | - Choose from the list - |
| | Cycle lanes | - Choose from the list - |
| Design | Traffic Load | - Choose from the list - |
| | Barrier performance level (AS 5100) | - Choose from the list - |
| | Exposure condition (AS 3600) | - Choose from the list - |
| Construction | Available Lead time (weeks) | |
| | Construction duration (Deck Only) (Days) | - Choose from the list - |
| Resource availability | Concrete | - Choose from the list - |
| | Crane | - Choose from the list - |
| | Special machinery | - Choose from the list - |

Column three of the table allows the selection of the best options or the ability to enter a value for the parameters listed in the second column to suit site requirements. An example of the site condition selection is shown in Figure 5.3. This will then generate a web diagram as shown in Figure 5.4 and a heatmap as shown in Figure 5.5.

Plots enclosed by the red line in the web diagram show the possible options for the replacement bridge, with the best option closest to the centre. Any point lying outside the red line indicates deviation from the site requirement. A deviating parameter can be identified from the red cells in the heat map. Some of these deviations may have only a minor effect, and it may be possible to modify the product easily to suit site conditions or adopt it with minor compromises. Hence, it is recommended to refer to the relevant section of the bridge system database for detailed information of such deviations.

Figure 5.3: Site condition screening – example

Site Condition Screening

| Category | Parameter | Site condition/ design requirement (Please select the best from the list/ Enter) |
|-----------------------|---|---|
| Site Accessibility | Maximum allowable GVM (tonne) | 60 |
| | Long vehicle access | Standard trucks (12.5 m) |
| | Accessibility for pre-works | Moderately Accessible |
| Bridge Componentry | Superstructure | New |
| | Substructure | Can use existing |
| | Foundation | Can use existing |
| Geometry | Span length (m) | 10 |
| | Number of traffic lanes | Two |
| | Foot path | None |
| | Cycle lanes | None |
| Design | Traffic Load | SM1600 |
| | Barrier performance level (AS 5100) | Regular |
| | Exposure condition (AS 3600) | B2 |
| Construction | Available Lead time (weeks) | 8 |
| | Construction duration (Deck Only) (Days) | Urgent (< 3 days) |
| Resource availability | Concrete | Limited |
| | Crane | 50 t |
| | Special machinery | Available |

Figure 5.4: Web diagram

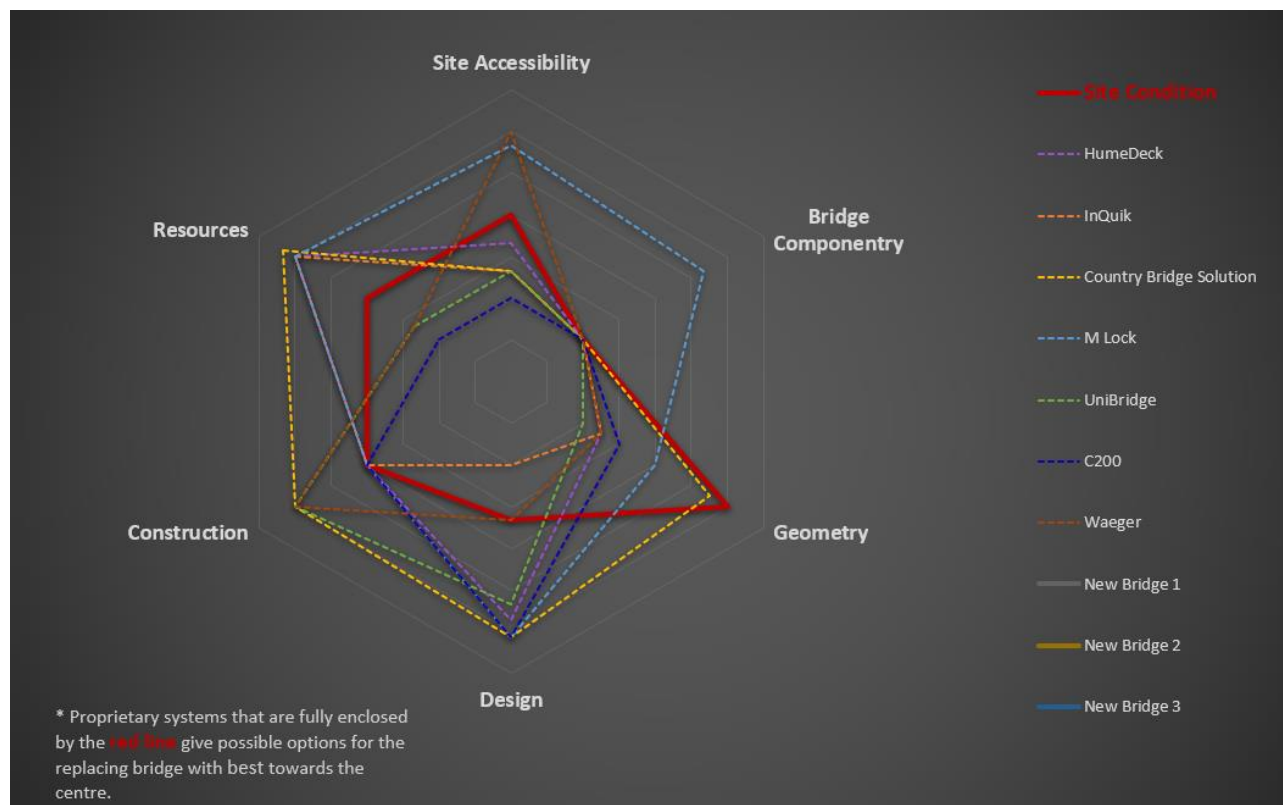


Figure 5.5: Heat map

| Category | Parameter | Site Condition | HumeDeck | InQuik | CBS | M Lock | UniBridge | C200 | Waeger |
|-----------------------|--|--------------------------|-------------------------|-------------------------|-------------------------|------------------------|-------------------------|-------------------------|-------------------|
| Site Accessibility | Heavy Vehicle access GVM (tonnes) | 60 | 50 | 25 | 45 | 40 | 26 | 20 | 50 |
| | Long vehicle access | Standard trucks (12.5 m) | Standard trucks (12.5m) | Standard trucks (12.5m) | Standard trucks (12.5m) | Semitrailer (19m) | Standard trucks (12.5m) | Standard trucks (12.5m) | Semitrailer (19m) |
| | Accessibility for pre-works | Moderately Accessible | Not required | Not required | Not required | Not required | Not required | Not required | Not required |
| Bridge Componentry | Superstructure | New | New | New | New | New | New | New | New |
| | Substructure | Can use existing | Can use existing | Can use existing | Can use existing | New | Can use existing | Can use existing | Can use existing |
| | Foundation | Can use existing | Can use existing | Can use existing | Can use existing | New | Can use existing | Can use existing | Can use existing |
| Geometry | Max Span length (m) | 10 | 12 | 18.5 | 12 | 15 | 44.8 | 61 | 15 |
| | Number of traffic lanes | Two | More than 3 | More than 3 | Two | Three | More than 3 | Two | More than 3 |
| | Foot path | None | Dedicated | Dedicated | None | Shared with cycles | Dedicated | Dedicated | Dedicated |
| | Cycle lanes | None | Dedicated | Dedicated | None | Shared with pedestrian | Dedicated | Dedicated | Dedicated |
| Design | Traffic Load | SM1600 | SM1600 | SM1600 | SM1600 | SM1600 | SM1600 | SM1600 | SM1600 |
| | Barrier performance level | Regular | Low | Medium | Low | Low | Low | Low | Regular |
| | Exposure condition | B2 | B2 | Marine | B1 | B1 | Marine | B1 | B2 |
| Construction | Available Lead time (weeks) | 8 | 8 | 6 | 10 | 8 | 12 | 6 | 16 |
| | Construction duration (Deck Only) (Days) | Urgent (< 3 days) | 1 | 2 | 2 | 2 | 2 | 2 | 3 |
| Resource availability | Concrete | Limited | Low | High | Moderate | Low | Low | Low | High |
| | Crane | 50 t | 150 | 50 | 125 | 100 | 50 | 0 | 50 |
| | Skilled Labour | Available | Not required | Not required | Not required | Required | Not required | Not required | Not required |

Colour Code

EXCEED

ACCEPTABLE

OUTSIDE CONDITIONS

5.3 Preliminary Selection

A matrix has been developed which lists a number of advanced parameters against bridge systems to further refine the outcomes from the first step. Parameters in the options selection matrix include:

- vendor
- componentry
- typical lengths
- width
- number of spans
- design loading
- design standard
- material
- duration of construction
- direct cost
- departures from TMR standard
- limitations.

The selection matrix is shown in Appendix A.

5.4 Final Selection

The final selection of the replacement bridge includes a detailed review of bridge system data which will help to better understand the design criteria and performance of the product. Bridge systems data sheets for bridges reviewed in this project are listed in the Appendix B.

5.5 Recommendations

The above proposed three-step selection process facilitates TMR in timber bridge replacement option selection with significant time saving. The data in decision-supporting tools are current at the time of writing. However, this tool should be updated to include improvements and developments of current products and should include any new products as they become available. Furthermore, parameters for screening have been selected to represent general site conditions. They should be updated and screened against the current network at the time of evaluation.

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APPENDIX A OPTION SELECTION MATRIX

Table A 1: Option selection matrix-part1

| Product | Vendor | Completeness | Lengths 0–10 m | Lengths 10–20 m | Lengths over 20 m | Width (m) | Number of lanes | Number of spans | Design life (years) | Design traffic load | Design standard | Superstructure material | Substructure material | Duration of construction (deck only) | In-line constructability | Direct cost (deck only) (per metre ²) |
|--|---|--|--------------------|------------------------------|---|---|--------------------|--|---------------------------|-------------------------------------|-------------------------------------|--|---|--|-----------------------------|---|
| HumeDeck Modular Bridge | Design and supply only – by Holcim (Australia) Pty Ltd | Foundation Substructure Superstructure Barriers | 6–10 | 10–12 | – | Any | Multilane | Multi-spans | 100 | SM 1600 | AS 5100 (2004) | Precast reinforced concrete | Precast reinforced or prestressed concrete | 1 day (decks with but joints) | Yes | Not available |
| InQuik Bridging System | Design and supply only – by InQuik Bridging System | Substructure (excluding foundation) Superstructure Barriers | 6.1 9.1 | 12.1 13.7 16.1 18.5 | – | 4.8 – 14.4 Custom widths | Multilane | Multi-spans | 100 | SM 1600 | AS 5100 (2017), Austroads (1992) | Reinforced concrete bridge with sacrificial galvanised steel, Magnelis or stainless steel formwork | Not included but compatible with any type of foundation | 2 days + time for in situ deck to gain strength | Yes | \$1100–1500 |
| Country Bridge Solution (CBS) | Design only – Roads and Maritime Services NSW | Design only | 8 10 | 12 | – | 4.2 6.5 | 1 or 2 | Multi-spans | 100 | SM 1600 | AS 5100 (2004) | Prestressed concrete | Reinforced concrete | 2 days + time for in situ stitch joints to gain strength | Yes | \$1750 (as in 2016) |
| M-Lock | Design and supply only– by Rocla Pty Ltd | Foundation Substructure Superstructure Barriers | 7.0 9.0 10.0 | 12 15 | – | 4.8 6.0 7.2 8.4 9.6 10.8 | Up to 3 lanes | Multi-spans | 100 | SM 1600 | AS 5100 (2004) | Precast reinforced concrete | Prestressed precast concrete piles | 2 days | Yes | \$2500–3500 |
| UniBridge | Design and supply of superstructure only – by UniBridge Australasia Pty Ltd | Superstructure only | | 11.4 17.4 | 22.8 28.8 34.2 40.2 | Any | Multilane | Multi-spans | 100 | SM 1600 and HLP 320 | AS 5100 (2004) | Steel girders + precast/in situ concrete deck | Not included but compatible with any type of foundation | 2 days | Yes | \$1200–1500 |
| C-200- Mabey | Fabrication and supply is by Mabey Australia (Fabricated in UK) | Superstructure only | | 15.24 18.2 | 21.3 - 60.96 in 3.048 increments | 3.15 4.20 7.35 | 1 or 2 | Can span full length in single span or with intermediate supports. | 100 | SM 1600 | AS 5100 (2004) | Steel | Not included but compatible with any type of foundation | 2 days | Yes | \$1800–2600 |
| Waeger | Design, supply and construction by Waeger Constructions Pty Ltd | Superstructure Substructure Foundation Barrier | 6–10 | 10–15 | – | Any | Multilane | Multi-spans | 100 | SM 1600 and HLP 321or HLP 400 | AS 5100 (2017) | Precast prestressed concrete | Precast concrete/in situ cast columns | 3 days | Yes | \$1250–2000 |

Table A 2: Option selection matrix-part 2

| Product | Departures from TMR standards | Limitations | Comments |
|--------------------------------------|--|---|---|
| HumeDeck Modular Bridge | <ol style="list-style-type: none"> Open expansion joints at deck No structural connection/load transfer between deck units when butt joints are used Use low performance W beams for barriers Barrier posts are mounted on the carriageway Relieving slab connection does not follow Standard drawing 2255. No allowance for rotation No provision for bearing replacement | <ol style="list-style-type: none"> In situ stitch joints require on-site formwork and concreting Unable to replace bearing pads. Hold down bolts are passing through elastomeric bearing pads Butt joints leave a gap of 10 mm between slab units. No structural connection or lateral load distribution between panels | Allowance for expansion of deck units are made at hold down bolts. Joints at deck level are open joints. |
| InQuik Bridging System | <ol style="list-style-type: none"> Typically, a fully integral structure; but can be provided with simply supported deck. If integrated, special approval may be required (Section 1.2.4 and 4.7.1 (c)-<i>Design Criteria for Bridges and Other Structures</i>, TMR 2018) No bearing replacement schedule (not required for integral bridges) | <ol style="list-style-type: none"> Foundation design is not included Requires large volume of in situ concrete For bridges longer than 12 m, deck beams need to be cast first and given time to gain sufficient strength before concreting the rest. This will increase the construction time | |
| Country Bridge Solution (CBS) | <ol style="list-style-type: none"> Low performance barrier Designed for B1 exposure conditions only Uses precast reinforced concrete driven piles No approach slab Narrow carriageway Not designed for wearing surface except 10 mm sprayed bituminous seal (Not complying with 4.8.3 of TMR 2018) | <ol style="list-style-type: none"> Design only Substructure has to be designed by contractor for site condition Not suitable to use within 1 km of the shoreline of salt water Maximum 2 traffic lanes and carriageway width is less than minimum recommended Maximum deck height of 10 m above ground/river bed level Equal spans only No dedicated pedestrian or cycle paths Maximum AADT of 1000 | No current supplier. Design is available for anyone to use. |
| M-Lock | <ol style="list-style-type: none"> Typically uses spun piles which are not allowed in QLD Deck units are bolted to headstock at both ends. No expansion joint or provision for rotation Designed for B1 (headstocks/decks) exposure conditions Uses oversized pile holes which are backfilled with sand/cement mix (Section 6 of MRTS65) Low performance Thrie beam bridge barriers bolted to side of the deck Uses steel, reinforced concrete piles which are restricted by TMR Typically, no kerbs, no scuppers. (Available on request) Uses prestressing bars to transversely prestress high traffic bridges which are left exposed (External post tension) No provision for relieving slabs | <ol style="list-style-type: none"> Longer construction time for substructure (typically 15 days per span) Precast bored piles require considerable amount of site work (driving steel casing, drilling, backfilling etc) Unable to replace bearing pads. Hold down bolts are going through the elastomeric bearing strips | <ol style="list-style-type: none"> Bolts can be used to transversely connect deck units. However, performance of bolts for transverse load distribution may be limited Bridges design for high traffic volumes use external transverse prestressing. Precast concrete barriers are sharing the same prestressing bars used to stress the deck. Damage to bridge barriers at an accident may release the tension in stressing bars |
| UniBridge | <ol style="list-style-type: none"> Superstructure only Not designed for scour, submergence Not designed for earthquake No kerbs on the deck Low performance barrier | Substructure is not included in the design | Earthquake design is required for bridges spanning over 20 m |
| C-200- Mabey | <ol style="list-style-type: none"> Narrow carriageway Typically, no barriers. But can be supplied. This may further reduce the carriageway width | <ol style="list-style-type: none"> Supply superstructure only Maximum 2 traffic lanes | <ol style="list-style-type: none"> Trusses are the main structural elements which are positioned at the same level as the road level but not designed for vehicle impact Separate barriers can be provided but may further reduce the carriageway width |
| Waeger | Typically, none | <ol style="list-style-type: none"> Maximum 15 m span 15 m decks are 26 t and over Typically, bespoke with some cast in situ elements such as pier columns | |

APPENDIX B DATA SHEETS

B.1 HumeDeck

Table B 1: HumeDeck bridge system data

| | Parameter | | Structure design criteria | Comments |
|--------|------------------------|----------------------------|--|----------|
| System | Developed by | Holcim (Australia) Pty Ltd | | |
| | Supplied by | Holcim (Australia) Pty Ltd | | |
| | Construction | | | |
| | Key features | | <ol style="list-style-type: none"> 1. Can be installed on existing or new substructure 2. Minimal longitudinal joints leading to faster installation 3. Deck units consist of cast into galvanized channel at ends to prevent spalling 4. Option of butt joint or in situ joints for longitudinal joints | |
| | Advantages | | <ol style="list-style-type: none"> 1. Fully precast option 2. Fast installation 3. Can be attached to existing headstock/abutments 4. Reduced site work | |
| | Limitations/weaknesses | | <ol style="list-style-type: none"> 1. On-site formwork and concreting are required if in situ joints are used 2. Hold down bolts are passing through the elastomeric bearing pads 3. If butt joint for slab units is selected, a gap of 10 mm between slab units will remain. No lateral load distribution between panels 4. Low performance barrier | |

| | Parameter | Structure design criteria | Comments |
|-----------------|--|---|---|
| General | Pre-engineered and standardized? | YES | |
| | Superstructure | YES | |
| | Substructure | YES | |
| | Barriers | YES – Can be custom designed to meet project specific requirements | |
| | Drain | YES – Allowance can be made for drainage requirements | |
| | Completeness of design (Y/N) | | |
| | Kerbs | YES | |
| | Bearing | YES – Can be custom designed to meet project specific requirements | |
| | Expansion joints | YES | |
| | Provision for services | Can be custom designed to meet project specific requirements | |
| | Standardised/pre-engineered components | Deck units, headstock, piles, abutments | |
| | In situ cast elements | In situ horizontal joints (stitch joint), connections of pile and headstock/abutment | |
| | Previously used/tested in Australia? | YES – MacArthur Gardens bridge in South Sydney, Koombuloomba Dam bridge, QLD etc. | |
| Structural form | Availability of documentation | YES – Humes provides project-specific shop drawings for full approval and installation manual | |
| | Room for future expansions | YES – A completely modular system can be lengthened. Can also be widened; however, would require new abutments and headstocks | |
| | Deck | 6 m to 12 m long and up to 2.7 m wide deck panels | |
| | Abutments | Reinforced concrete precast abutment on piles with in situ joint | |
| | Pier | Precast reinforced concrete headstock on 550 mm x 550 mm rectangular precast or 450 mm to 550 mm octagonal prestressed piles | |
| | Headstock | Custom-made precast reinforced concrete units in situ jointed to piles or bolted to existing piers | |
| | Foundation | Precast rectangular reinforced concrete or prestressed octagonal piles (potted or driven) or use existing foundation | |
| | Wing walls | Precast reinforced concrete units. Can be custom-designed to meet project-specific requirements | |
| | Relieving slab | Precast relieving slab connected to bridge with in situ stitch | Connection does not follow Standard drawing 2255. No allowance for rotation |

| | Parameter | Structure design criteria | Comments |
|-------------------|---|--|---|
| Primary materials | Superstructure | Precast reinforced concrete | Hold down bolts are passing through the bearing pad hence may not to be replaced |
| | Substructure | Precast reinforced concrete or prestressed concrete piles with precast reinforced concrete headstock/abutment | |
| | Bearing | Typically, elastomeric sacrificial bearing pads | |
| | Barriers | Steel W beams (on concrete or kerb if required) | |
| | Expansion joints | Elastomeric sealant around hold down bolts. Open at the deck level | |
| | | | |
| Design | Design standards | AS 5100 (2004) | |
| | Design traffic load | SM1600 | |
| | Other design forces | Earthquake Submergence Scour Barrier impact Other | |
| | | NO YES YES – Flow velocity of 4 m/s YES – Low performance barrier | |
| | | Super imposed dead load includes – 230 mm asphalt layer over the entire bridge and 100 mm concrete topping to pedestrian walkways | |
| | Design life | 100 years | |
| | Design AADT | Not specified | |
| | Design speed | Not specified | |
| | Other design assumptions | Not specified | |
| | Elements not covered in standard design | Geotechnical | |
| Compliance | Compliance to <i>Design Criteria for Bridges and Other Structures</i> | Typical deviations are listed below | Clause 4.11 – DCBOS Standard drawing 2255 Clause 4.6.2 – DCBOS Clause 4.9.5.1, Clause 4.9.5.1 – DCBOS Clause 4.9.4 – DCBOS Clause 4.10.2 – DCBOS |
| | Compliance to TMR specifications | Typical deviations are listed below | |
| | Departures from TMR specifications | 1. Relieving slab connection does not follow Standard drawing 2255. No allowance for rotation 2. No provision for bearing replacement 3. Use low performance barriers with W beams 4. Bridge rails are mounted on the carriageway 5. Open expansion joints | |
| Geometry | Typical widths | Any width with panel widths up to 2.7 m | |
| | Typical span length | 6 m – 12 m | |
| | Number of spans | Multiple | |
| | Number of traffic lanes | Multilane | |
| | Pedestrian/cycle lanes | As required – Can be custom designed to meet project-specific requirements | |

| | Parameter | Structure design criteria | Comments |
|---------------------|--|--|--|
| Site considerations | Adaptability for various site conditions Skew Grade Horizontal curve Vertical curve Required site investigations Resistance for corrosive environments (design exposure condition) | YES. Up to 20° YES NO NO Geotechnical B2 Exposure | |
| Constructability | In-line construction Shut down option (full shut/lane shut) Deployment lead time Availability of prefabricated/precast items Special machinery Special skills/techniques required at site Transportability of precast components Weight of heaviest part Size of largest part Time to complete deck | YES Usually shut down existing bridge 8 weeks on average Australia-wide Pile driving machines None Standard vehicles 29.03 t (for 12 m span) 12 m x 2.7 m x 0.9 m 24 hours for deck | Installing deck with butt joints only. Time for in situ joints has to be considered for other type |
| Cost | Approximate construction cost per sqrm Approximate annual maintenance cost | Information not available Not specified | |
| Maintenance | Recommended inspection frequency Recommended routine maintenance Durability | Annual As required 100 years design life/B2 exposure | |
| Considerations | | 1. If butt joint for slab units is selected, a gap of 10 mm between slab units will remain 2. No lateral load distribution between panels 3. Typically, bridge rails are mounted on the carriageway | |
| References | Humes modular Bridge system website Hume deck system -Brochure | https://www.holcim.com.au/humes/precast-concrete-solutions/bridge-and-platform/modular-bridge-system https://www.holcim.com.au/sites/australia/files/atoms/files/hu-humedeck-modular-bridge-system-iss1.pdf | |

B.2 InQuik

Table B 2: InQuik bridge system data

| | Parameter | | Structure design criteria | Comments |
|--------|------------------------|---------------------------|---|---|
| System | Developed by | InQuik and SMEC Australia | | Fabricated by Australian Reinforcing Company (ARC) |
| | Supplied by | InQuik Bridging Systems | | |
| | Construction | | | |
| | Key features | | <ol style="list-style-type: none"> 1. A semi-modular system integrated with prefabricated sacrificial steel formwork and reinforcing components 2. InQuik abutments, wing walls, headstocks and blade piers are constructed using a similar semi-modular method as the deck 3. Greater adoptability to site conditions with several options for major components 4. Certified by SMEC Australia Pty Ltd (SMEC) to the Australian Standards, AS 5100 (2017) Bridge Design requirements 5. Option of integral or simply supported deck | |
| | Advantages | | <ol style="list-style-type: none"> 1. No temporary formwork/supports needed 2. Can integrate with all major foundation types 3. Lightweight prefabricated items 4. No special machinery and skill required 5. Permanent steel formwork provides additional protection and durability to concrete | No exposed concrete surfaces. Visual inspection may not detect concrete defects |
| | Limitations/weaknesses | | <ol style="list-style-type: none"> 1. Prior approval may require for fully integral structure (Section 1.2.4 – <i>Design Criteria for Bridges and Other Structures</i>, TMR 2017) 2. Foundation design is not included 3. Requires large volume of in situ concrete 4. Deck needs concreting in 2 states for over 12 m long deck panels. (Beams to be cast first and wait for gaining strength before concreting rest) 5. Max abutment height – 2.4 m in a single pour | |

| | Parameter | Structure design criteria | Comments |
|---------|--|---|---|
| General | Pre-engineered and standardized? (Y/N) | YES | |
| | Superstructure | YES | |
| | Substructure | Abutments, wing walls, blade piers and headstocks are provided by InQuik. Foundation to be designed by the contractor | All foundation types can be used with the InQuik system |
| | Deck | YES | Standard deck panel width is 2.4 m wide, lay panels side by side for wider deck. Can also increase deck width using extension spacers between deck panels, up to 550 mm each. Also offer 3.2 m-wide panels for narrow bridges (e.g. for farms, service roads, etc). |
| | Completeness of design (Y/N) | | |
| | Barriers | YES | Low (side/top-mounted), regular and medium performance |
| | Drain | YES | Deck can have cross-fall, and if necessary castellated kerbs/scuppers can be provided |
| | Kerbs | YES | Castellated or continuous with drainage scuppers |
| | Bearing | YES – if required | The standard InQuik single span bridge is fully integral, removing bearings and tiedowns |
| | Expansion joints | YES – if required | |
| | Provision for services | YES – if required | Provision for 3 x 165 mm service holes can be incorporated into each panel |
| | Standardised/pre-engineered components | Deck panels, abutments, headstocks, wing walls, blade piers or fully integral bridge structure | |
| | In situ cast elements | All components are FULLY cast in situ | This means there is one mass of concrete on the deck, and thus there are no deck joints, grout, tie bolts, and no surface treatment is required |
| | Previously used/tested in Australia? | YES – previous projects in NSW: Snowy Mountains (Murrumbidgee River, 12 m single lane with 15 degree skew), Boxers Creek (12 m single lane), Bellingen Council (Reids Creek, 12 m double lane), Hills Shire Council (Caddies Creek, 12 m single lane with service holes), Chivers Bridge (13.7 m single lane, fully integral), James Bridge (9 m single lane, fully integral), Byron Council (Durrumbal Causeway, 16.1 m single lane, fully integral), Snowy Monaro Council (Merima Rd, 12 m single lane, fully integral) | |

| | Parameter | Structure design criteria | Comments |
|------------------|--|--|--|
| | Availability of documentation Room for future expansions | Case studies, product catalogue, technical documentation, certifications, install guides, etc. all available YES – If required. | |
| Structural form | Deck Abutments Pier Headstock Foundation Wing walls Bearing | Fully reinforced in situ cast concrete structure with permanent sacrificial steel formwork (protects concrete) Fully reinforced in situ cast concrete abutments with permanent sacrificial steel formwork (protects concrete) Fully reinforced concrete pier with left in place sacrificial steel formwork Fully reinforced in situ cast concrete headstock with permanent sacrificial steel formwork (protects concrete) Compatible with any type of foundation Cantilevered wing walls off main abutment – fully reinforced concrete structure Elastomeric strip bearing if designed for it – Usually fully integral structure requiring no bearings | To be designed by contractor |
| Primary material | Superstructure Substructure Bearing Barriers Expansion joints | Reinforced concrete bridge with sacrificial galvanised steel, Magnelis or stainless steel permanent formwork Abutments, headstocks and blade piers are reinforced concrete with sacrificial Magnelis/stainless steel permanent formwork with any type of foundation Elastomeric strip bearing if designed for it (fully integral structure requires no bearings) Steel or concrete Optional | Magnelis- is an exceptional metallic coating with very high corrosion resistance |
| Design | Design standards Design traffic load Other design forces Earthquake Submergence Scour Barrier impact Other Design life Design AADT Design speed Other design assumptions Elements not covered in standard design | AS 5100 (2017), Austroads (1992) SM1600, T44/62 t B-double No No Flow velocity 2 m/s Low, regular and medium – 100 Years No limitation As required Not specified Foundation | Deck panels can include anti-buoyancy air vents |

| | Parameter | Structure design criteria | Comments |
|---------------------|---|---|--|
| Compliance | Compliance to <i>Design Criteria for Bridges and Other Structures</i> | Typical deviations are listed below | |
| | Compliance to TMR specifications | Typical deviations are listed below | |
| | Departures from TMR specifications | 1. Normally a fully integral structure. Special approval may be required (Section 1.2.4 – <i>Design Criteria for Bridges and Other Structures</i> , TMR 2017) 2. Bearing replacement schedule to be determined (not required for integral bridges) | Clause 1.2.4 – DCBOS Clause 4.6.2 – DCBOS |
| Geometry | Typical widths | 4.8 m – 14.4 m or custom widths | |
| | Typical span length | 6.1 m to 18.5 m (6.1, 9.1, 12.1, 13.7, 16.1, 18.5) | |
| | Number of span | Multi-spans | |
| | Number of traffic lanes | Multilane | |
| | Pedestrian/cycle lanes | Can provide if required | |
| Site considerations | Adaptability for various site conditions | Skew Grade Horizontal curve Vertical curve | |
| | Required site investigations | Geotechnical, site survey and flood data | |
| | Resistance for corrosive environments (design exposure condition) | 40 MPa for B1 Classification, 50 MPa for B2, and marine concrete + thicker cover for C classification | |
| | | | |
| | | | The formwork protects the concrete from corrosion for the life of the forms – If Magnelis is used, the formwork coating alone will last over 100 years |

| | Parameter | Structure design criteria | Comments |
|------------------|---|--|---|
| Constructability | In-line replacement | Requires side track or full road closure (steam curing or high early strength concrete can be used to shorten closure time) | <p>*Abutments typically take 1 day to install and concrete, then the decks take a few hours to install, and concrete is usually poured the following day.</p> <p>*Deck needs concreting in 2 stages for over 12 m long deck panels. (Beams to be cast first and wait for gaining strength before concreting rest). This will increase construction time</p> |
| | Shut down option (full shut/lane shut) | Full shut during full construction period | |
| | Deployment lead time | Typically, 4–6 weeks | |
| | Availability of prefabricated/precast items | Components are sometimes available ex stock, but short lead times can be met | |
| | Special machinery | No special machinery to install | |
| | Special skills/techniques required at site | No | |
| | Transportability of precast components | No precast parts | |
| | Weight of heaviest part | Depends on the length of the unit – 12 m x 2.4 m deck weighs ~4.5 tonne. Single lane abutment is typically ~1.2 tonne | |
| | Size of largest part | 18.5 m x 2.4 m deck weighs ~9.5 tonne | |
| Cost | Time to complete deck | 2 days to install and concrete deck panels. Time required for gaining strength | |
| | | | |
| Cost | Approximate construction cost per sqrm | Whole-of-job construction cost can typically be \$2000 to \$3500 per sqrm, depending on the foundation type, site conditions, barrier designs etc. Approximately \$1100 – \$1500 per sqrm for slab only | No bearing, no exposed concrete. Only barrier and surfacing maintenance |
| | Approximate annual maintenance cost | Minimum maintenance if integral structure is used | |
| Maintenance | Recommended inspection frequency | Not specified | |
| | Recommended routine maintenance | Not specified | |
| | Durability | 100 years of design life | |
| Considerations | | Requires a large volume of concrete No exposed concrete surfaces. Visual inspection may not detect any concrete defects | |
| References | InQuik website | http://inquik.com.au/ | |
| | InQuik Bridging" Brochure | http://inquik.com.au/wp-content/uploads/2017/08/Trifold-v2.3.5.pdf | |
| | InQuik Catalog | http://inquik.com.au/wp-content/uploads/2016/10/Inquik-Catalogue-v1.0.5.pdf | |

B.3 Country Bridge Solution

Table B 3: CBS bridge system data

| | Parameter | | Structure design criteria | Comments |
|--------|------------------------|---------------------------------|---|---------------------------------------|
| System | Developed by | Road and Maritime Services, NSW | | Design is available for anyone to use |
| | Supplied by | No current supplier | | |
| | Construction | | | |
| | Key features | | <ol style="list-style-type: none"> 1. Designed to Australian standard- AS 5100 (2017) 2. A fully certified bridge deck system, incorporating prestressed concrete double-T deck modules 3. Standardised substructure components can be easily adapted to suit a range of site conditions 4. Three standard bridge configurations to suit different future traffic demands and site constraints 5. A suite of standard bridge drawings for a modular bridge solution | |
| | Advantages | | <ol style="list-style-type: none"> 1. Available for anyone to manufacture 2. Transportability of precast components on standard trucks 3. Minimised on-site concrete required 4. Reduced maintenance requirements resulting from a 100 years design life | |
| | Limitations/weaknesses | | <ol style="list-style-type: none"> 1. Design only; Available for anyone to construction 2. Substructure has to be designed by the contractor for site condition 3. Designed for exposure condition B1 only. Not suitable to use within 1 km of the shoreline of salt water 4. Design for equal spans only 5. Maximum 2 traffic lanes and AADT of 1000 6. Maximum deck height of 10 m above the ground (limited column height) 7. No dedicated pedestrian or cycleway 8. No relieving slab | |

| | Parameter | Structure design criteria | Comments |
|------------------|--|---|---|
| General | Pre-engineered and standardized? (Y/N) | YES | The contractor design foundation for site conditions. Standard options are available to adopt |
| | Superstructure | YES | |
| | Substructure | NO | |
| | Completeness of design (Y/N) | | |
| | Barriers | YES – Low performance barrier | |
| | Drain | Scuppers can be included in deck modules | |
| | Kerbs | Included in the external deck module | |
| | Bearing | YES | |
| | Expansion joints | YES – Pour in place seal joint | |
| | Provision for services | NO | |
| Structural form | Standardised/pre-engineered components | Deck module, traffic barriers, bearing, holding downs and restrain brackets, pier headstock, abutment sill beam, wing walls | |
| | In situ cast elements | Deck closure strips, pile-headstock connections | |
| | Previously used/tested in Australia? | Trial bridge at Bookookoorara Creek, NSW | |
| | Availability of documentation | Full documentation available for entire assets life cycle | |
| | Room for future expansions | Not considered | |
| | | | |
| Primary material | Deck | Precast prestressed concrete double-T deck units + in situ closure pour | |
| | Abutments | Precast sill beam on pile/column supports | |
| | Pier | Pile/column on spread footing depending on site conditions | |
| | Headstock | Precast concrete unit on pile/column supports with in situ connection | |
| | Foundation | Spread footing/precast driven piles | |
| | Wing walls | Precast concrete units | |
| Primary material | Superstructure | Prestressed precast concrete | |
| | Substructure | Precast reinforced concrete | |
| | Bearing | Laminated elastomeric bearings – replaceable | |
| | Barriers | Steel low performance barriers | |
| | Expansion joints | Pore in place sealant | |

| | Parameter | Structure design criteria | Comments |
|---------------------|---|---|--|
| Design | Design standards | AS 5100 (2004), Austroads bridge design code (1992) | |
| | Design traffic load | SM1600, T44 (single-lane bridge) | |
| | Other design forces | Earthquake Submergence Scour Barrier impact Other | YES YES – 5 m overtopping Water flow scour (flow velocity 4 m/s) YES – Low performance barrier Vehicle braking forces |
| | Design life | 100 Years | |
| | Design AADT | Max AADT 1000 vehicles per day | |
| | Design speed | Not specified | |
| | Other design assumptions | Equal spans only, no wearing surface except 10 mm sprayed bituminous seal | |
| | Elements not covered in standard design | Everything below the headstock has not been included in standard documents and has to be designed for site conditions. E.g. piles, pile caps, columns, footings, temporary supports, connections | |
| | Compliance to <i>Design Criteria for Bridges and Other Structures</i> | Typical deviations are listed below | |
| | Compliance to TMR specifications | Typical deviations are listed below | |
| Compliance | Departures from TMR specifications | <ol style="list-style-type: none"> 1. Low performance barrier 2. B1 exposure conditions 3. Use precast reinforced concrete driven piles 4. No relieving slab 5. Narrow carriageway 6. Not designed for wearing surface except 10 mm sprayed bituminous seal (Not complying with 4.8.3 of DCBOS) | Clause 4.9.1.1 – DCBOS Clause 10.4.5 – DCBOS Clause 4.1.1 – DCBOS Clause 4.11 – DCBOS Clause 3.1 – DCBOS Clause 4.8.3 – DCBOS |
| | | | |
| Geometry | Typical widths | Min 4.2 m for single lane, 6.5 m for 2 lanes | |
| | Typical span length | 8 m, 10 m, 12 m | |
| | Number of spans | Multi-spans (equal spans only) | |
| | Number of traffic lanes | Maximum 2 lanes | |
| | Pedestrian/cycle lanes | No | |
| Site considerations | Adaptability for various site conditions | Skew Grade Horizontal curve Vertical Curve | No Yes. Constant grades only No No |
| | Required site investigations | Geotechnical | |
| | Resistance for corrosive environments (design exposure condition) | Designed for B1 exposure category | |
| | | | |
| | | | |

| | Parameter | Structure design criteria | Comments |
|------------------|--|--|---|
| Constructability | In-line construction | YES | Side track or full bridge closure is required |
| | Shut down option (full shut/lane shut) | Full shut | |
| | Deployment lead time | No current supplier | |
| | Availability of prefabricated/precast items | No current supplier. Open for anyone to use | |
| | Special machinery | One large crane, machine for driving piles | |
| | Special skills/techniques required at site | None | |
| | Transportability of precast components | Can be transported in a standard truck | |
| | Weight of heaviest part | 23.2 tonnes | |
| | Size of largest part | 2.06 m x 9.97 m x 0.755 m | |
| | Time to complete deck | 2 days. Time required for stitch joints to gain strength | |
| Cost | Approximate construction cost of deck per sqrm | \$1750 per sqm | |
| | Approximate Annual Maintenance cost | Not specified | |
| Maintenance | Recommended inspection frequency | Level 1 inspections at every 6 months, Level 2 inspection at every 2 years | |
| | Recommended routine maintenance | As required | |
| | Durability | Designed for B1 exposure category and 100 years of design life | |
| References | http://www.rms.nsw.gov.au/business-industry/partners-suppliers/lgr/country-bridge-solutions.html All documents and drawings are available to download from | | |

B.4 M Lock

Table B 4: M Lock bridge system data

| | Parameter | | Structure design criteria | Comments |
|--------|------------------------|---------------|--|--|
| System | Developed by | Rocla Pty Ltd | | Rocla temporarily ceased the sale of M-Lock on 01/05/2018 until further notice |
| | Supplied by | Rocla Pty Ltd | | |
| | Construction | | | |
| | Key features | | <ol style="list-style-type: none"> 1. A complete bridge system 2. Designed in accordance with AS 5100 (2004), for SM1600 traffic loadings, with a design life of major components of 100 years 3. Cardno design certification of precast components and system design 4. Design Options – Cost effective 'Bolted' assembly for < 1000 AADT bridges, 'Post-Tensioned' system for > 1000 AADT | |
| | Advantages | | <ol style="list-style-type: none"> 1. No large in situ concretes. (No ready-mix concrete is required) 2. Span over span construction (can be constructed by reaching out) 3. Minimal construction impact on waterways 4. Less on-site skilled labour requirement | |
| | Limitations/weaknesses | | <ol style="list-style-type: none"> 1. Longer construction time for substructure (typically 15 days per span) 2. Restricted use of available pile types in QLD 3. Precast bored piles require considerable amount of site work (driving steel casing, drilling, backfilling etc.) 4. Deck units are fitted to headstock using bolts. No provision of expansion joints may results limitations in use 5. Designed to B1 (headstocks/decks) exposure conditions 6. Performance of bolts for transverse load distribution and durability may be limited 7. Standard design does not consider scour or submergence | |

| | Parameter | Structure design criteria | Comments |
|-----------------|--|--|---|
| General | Pre-engineered and standardized? (Y/N) | YES | <p>Low traffic bridges – Low performance steel thrie beam barriers bolted to side of the deck panels</p> <p>High Traffic bridges – precast medium performance barriers fixed through prestressing bars on side of deck</p> <p>Drainage through decks. Can supply scupper holes in kerbing if required</p> <p>Typically, no kerb</p> |
| | Superstructure | YES | |
| | Substructure | YES | |
| | Barriers | YES | |
| | Completeness of design (Y/N) | | |
| | Drain | NO | |
| | Kerbs | Optional | |
| | Bearing | Elastomeric strip | |
| | Expansion joints | NO | |
| | Provision for services | Optional | |
| | Standardised/pre-engineered components | Piles, headstock, plank, end protection beams | |
| | In situ cast elements | No in situ concrete | |
| | Previously used/tested in Australia? | Yes – first bridge – over Ohio Creek at Walcha, NSW (1995) + over 230 bridges in Australia | |
| Structural Form | Availability of documentation | Technical guides, installation guides, Cardno design certification | <p>Deck units are transversely connected using M20 bolts. A high tensioned prestress bar is used for transverse connection for bridges with high volume of traffic (Over 1000 AADT)</p> <p>Typically, bored hollow piles, backfilled with cement/sand mix and pile void filled with stabilized sand</p> |
| | Room for future expansions | Dependant on applications | |
| | Deck | Precast concrete inverted U sections fitted to headstock using bolts | |
| | Abutments | Precast headstock on piles+ Rocla MassBloc wall to retain soil | |
| | Pier | Precast headstock on piles | |
| | Headstock | In situ jointed precast concrete unit | |
| | Foundation | Socketed hollow precast piles, square RC piles, steel UC or tubular piles | |
| | Wing walls | Rocla MassBloc | |

| | Parameter | Structure design criteria | Comments |
|------------------|---|---|---|
| Primary material | Superstructure | Precast reinforced concrete | Transversely prestressed deck for high traffic bridges Sacrificial bearings. Unable to lift for bearing replacement due to grouted hold down bolts |
| | Substructure | Precast reinforced concrete | |
| | Bearing | Elastomeric strip | |
| | Barriers | Thrie beam bolted on barrier rail or precast concrete crash barrier | |
| | Expansion joints | No | |
| Design | Design standards | AS 5100 (2004) | |
| | Design traffic load | SM1600 | |
| | Other design forces | Earthquake | |
| | | Submergence | |
| | | Scour | |
| | | Barrier Impact | |
| | Design life | 100 years | |
| | Design AADT | Options of less than 1000 or more than 1000 | |
| | Design speed | As required | |
| | Other design assumptions | – | |
| | Elements not covered in standard design | – | |
| Compliance | Compliance to <i>Design Criteria for Bridges and Other Structures</i> | Typical deviations are listed below | Clause 4.1.6, Clause 4.3 – DCBOS Clause 10.4.5 – DCBOS Section 6 – MRTS65 Clause 4.9.1.1, Clause 4.9.5.1 – DCBOS Clause 4.6.2 – DCBOS Clause 4.7.2 – DCBOS Clause 4.11 – DCBOS Standard Drawing 2255 |
| | Compliance to TMR specifications | Typical deviations are listed below | |
| | Departures from TMR specifications | 1. Typically, uses spun piles, steel or RC piles which are not allowed in QLD | |
| | | 2. B1 (headstocks/decks) exposure conditions | |
| | | 3. Uses oversized piles holes which are backfilled later with sand/cement mix (Violates MRTS65 section 6) | |
| | | 4. Low performance Thrie beam bridge barriers bolted to side of the deck (low traffic bridges) | |
| | | 5. Hold down bolts are going through the bearing strip. May not be replaceable | |
| | | 6. Uses external prestressing bars to transversely prestress high traffic bridges | |
| | | 7. No provision for relieving slabs | |

| | Parameter | Structure design criteria | Comments |
|-------------------------------------|---|---|---|
| Geometry | Typical widths | 4.8 m, 6.0 m, 7.2 m, 8.4 m, 9.6 m, 10.8 m | |
| | Typical span length | 7 m – 15 m | |
| | Number of spans | Single/multi-spans | |
| | Number of traffic lanes | Up to 3 lanes | |
| | Pedestrian/cycle lanes | Optional | |
| Site considerations | Adaptability for various site conditions | Skew Grade Horizontal curve Vertical curve | 30° skew angles are available up to 5% longitudinally, 7% cross-fall No No |
| | Required site investigations | Geotechnical | |
| | Resistance for corrosive environments (design exposure condition) | Headstocks/decks- B1, piles B2 exposure | |
| | In-line construction | YES | |
| | Shut down option (full shut/lane shut) | Full shut | |
| Constructability | Deployment lead time | 6–8 weeks lead time typically | |
| | Availability of prefabricated/precast items | 6–8 weeks lead time typically | |
| | Special machinery | Drilling rig for piles | |
| | Special skills/techniques required at site | Transverse post tensioning if required | |
| | Transportability of precast components | Standard length or extended length semi-truck | |
| | Weight of heaviest part | 15 m long post-tensioned deck plank – 20 tonnes 15 m long deck plank – 15 m long x 1.2 m wide x 0.8 m deep | |
| | Size of largest part | | |
| | Time to complete deck | 2 days | |
| Cost | Approximate construction cost per sqrm (deck only) | \$1000–1400 | |
| | Approximate annual maintenance cost | Not specified | |
| Maintenance | Recommended inspection frequency | Not specified | |
| | Recommended routine maintenance | Not specified | |
| | Durability | 100 years design life/ B1 exposure | |
| Special requirements/considerations | | <ol style="list-style-type: none"> 1. Temporary steel casing may be required for piles 2. Deck panels are fixed to the abutment/pier headstock at both ends. No allowance for expansion or rotation 3. No expansion joints. Gap between spans to be filled with non-shrinking mortar 4. Performance of bolts for transverse connection need to be assessed (e.g. durability, fatigue capacity) 5. No provision for relieving slab 6. Unable to lift for bearing replacement due to grouted hold down bolts which are also passing through bearing strip 7. External prestressing bars for transverse prestressing 8. Typically, no kerb or scuppers. (Available on request) | |

| | Parameter | Structure design criteria | Comments |
|------------|---------------|---|----------|
| References | Rocla website | http://www.rocla.com.au/M-Lock-Precast-Bridge.php | |

B.5 UniBridge

Table B 5: UniBridge system data

| | Parameter | | Structure design criteria | Comments |
|---------|--|-------------------------------|---|---|
| System | Developed by: | UniBridge Australasia Pty Ltd | | |
| | Supplied by: | UniBridge Australasia Pty Ltd | | |
| | Construction | | | |
| | Key features | | <ol style="list-style-type: none"> 1. All steel or composite options 2. Options of modular steel deck with anti-skid system, in situ cast or precast concrete deck 3. Structural members are underneath the deck level with no chance of vehicle collision or damage 4. Longer span up to 44.8 m 5. All connections are made with either pins or bolts for easy connection | |
| | Advantages | | <ol style="list-style-type: none"> 1. Can be completed without sophisticated equipment 2. Easy to expand 3. Can be launched using launching nose or a crane. Temporary supports are not required 4. Can be used as temporary or permanent structure 5. Specialist tools or on-site welding is not required | |
| | Limitations/weaknesses | | <ol style="list-style-type: none"> 1. UniBridge provides superstructure only 2. Not designed for scour, submergence 3. Not designed for earthquake 4. No kerbs 5. Low performance barrier | |
| General | Pre-engineered and standardized? (Y/N) | | YES | Supply typical design |
| | Superstructure | | YES | |
| | Substructure | | NO | |
| | Barriers | | YES | |
| | Completeness of design (Y/N) | Drain | YES | |
| | | Kerbs | NO | |
| | | Bearing | YES | |
| | | Expansion joints | No intermediate joints | |
| | | Provision for services | NO | |
| | Standardised/pre-engineered components | | Main box girders, deck – also barriers, barrier posts, bearings | |
| | In situ cast elements | | Reinforced concrete poured in situ deck on permanent steel formwork | |
| | | | | Steel or precast options are also available |

| | Parameter | Structure design criteria | Comments |
|------------------|--|--|--|
| | Previously used/tested in Australia? | YES – Gold Coast city council currently using as temporary bridge in bridge replacement projects. At the end of programme, same bridge will be installed as a permanent bridge. Temporary bridge on Kilcoy – Toogoolawah road | |
| | Availability of documentation/drawings Room for future expansions | Supplies general layout drawings for each and every bridge with all measurements true and correct Steel bridges can easily be expanded in width or moved and reconfigured as needed. The cast in situ or precast can also be expanded in width but this is more difficult to do because of the type of deck | |
| Structural form | Superstructure/deck | Basic module – 2 fabricated steel box girders + concrete-steel composite deck | Site-specific design by the contractor Site-specific design by the contractor |
| | Abutments | Supplies typical abutment designs (formwork and reinforcement) which the contractor can then adapt to the specific site and pile design based on the geotechnical reports | |
| | Pier | Supplies pier typical designs | |
| | Headstock | Supplies headstock typical designs | |
| | Foundation | Compatible with any type of foundation | |
| | Wing walls | | |
| Primary material | Superstructure | Steel + reinforced concrete | No intermediate joints. Freyssinet joints are used for the expansion joint between abutment and deck |
| | Substructure | Site-specific designed by the contractor | |
| | Bearing | Typically, neoprene bearings. However, steel pot or slide bearings can be provided | |
| | Barriers | Galvanised steel | |
| | Expansion joints | Not required | |

| | Parameter | Structure design criteria | Comments |
|---------------------|---|---|--|
| Design | Design standards | AS 5100 (2004) | Earthquake design may be required for over 20 m long bridges |
| | Design traffic load | T44, SM 1600, HLP320 | |
| | Other design forces | No | |
| | Earthquake | No | |
| | Submergence | No | |
| | Scour | No | |
| | Barrier impact | Low performance barriers | |
| | Other | Fatigue life of 1 000 000 (one million) cycles (AS 5100) | |
| | Design life | 100 years | |
| | Design AADT | Not specified | |
| | Design speed | Not specified | |
| Compliance | Other design assumptions | Not specified | Clause 4.9.1.1, Clause 4.9.5.1 – DCBOS |
| | Elements not covered in standard design | Substructure | |
| | Compliance to <i>Design Criteria for Bridges and Other Structures</i> | Generally, complies | |
| | Compliance to TMR specifications | Generally, complies | |
| Geometry | Departures from TMR specifications | Low performance barriers | |
| | Typical widths | Cast in situ or precast concrete decks can be anything. All steel: one lane: 4 m, two lanes: 7 m, three lanes: 10 m, four lanes: 13 m etc | |
| | Typical span length | Single lane 11.4 m – 50.8 m, Double lanes 11.4 m – 44.8 m (clear spans) | |
| | Number of spans | Multi-spans | |
| | Number of traffic lanes | Multilane | |
| Site considerations | Pedestrian/cycle lanes | Can provide if required | |
| | Adaptability for various site conditions | NO | |
| | Skew | YES | |
| | Grade | NO | |
| | Horizontal curve | NO | |
| Site considerations | Vertical curve | NO | |
| | Required site investigations | Geotechnical | |
| | Resistance for corrosive environments (design exposure condition) | Unless in a marine environment, use C4 level of paint protection. In a marine environment C5 marine level coating is used | |

| | Parameter | Structure design criteria | Comments |
|------------------|--|---|---------------------|
| Constructability | In-line construction Shut down option (full shut/lane shut) Deployment lead time Availability of prefabricated/precast items Special machinery Special skills/techniques required at site Transportability of precast components Weight of heaviest part Size of largest part Time to complete (single span, 2 lane bridge) | YES Full shut 90 days from order to reach nearest port (Brisbane) Not available in stock. Imported from France No No Can be transported by semi-trailers Varies with bridge size Varies with bridge size 2 days (superstructure with steel deck) | Imports from France |
| Cost | Approximate construction cost per sqm (deck only) | \$1100 – \$1500 | |
| Maintenance | Recommended inspection frequency Recommended routine maintenance Durability | Annually Annually 100 years design life/C4 exposure | |
| Considerations | | | |
| References | UniBridge website http://www.unibridge.net.au/ | | |

B.6 C200

Table B 6: C200 Bridge system data

| | Parameter | | Structure design criteria | Comments |
|---------|--|----------------------|---|--|
| System | Developed by: | Mabey Bridge Limited | | |
| | Supplied by: | Mabey Australia | | |
| | Construction | | | |
| | Key features | | <ol style="list-style-type: none"> 1. Modular and pre-engineered 2. Separate pedestrian walkways can be mounted 3. Can be assembled by hand or light plant 4. Various deck options are available 5. Transportable in a standard container 6. Optional ramps, piers, foot walks, parapets and barriers | |
| | Advantages | | <ol style="list-style-type: none"> 1. Easy to transport 2. Can be used as temporary, permanent or emergency bridge 3. Rapidly assembled and installed 4. Longer unsupported spans (Up to 60.96 m) 5. Crane installation or cantilever launch | |
| General | Limitations/weaknesses | | <ol style="list-style-type: none"> 1. Maximum 2 traffic lanes 2. Superstructure only. Contractor designs the substructure 3. Narrow carriageway 4. Typically, no barriers. But can be attached if required | C 200 trusses are not designed to act as a traffic barrier. Separate barriers can be attached |
| | Pre-engineered and standardized? (Y/N) | | Yes – The compact bridge system uses standard, interchangeable steel components | <p>The kerb is a formed fabricated plate, which is bolted to the side of the deck units, which form the edge of the roadway and are located close to the bridge trusses</p> <p>The bearing baseplate supports the bridge bearings under each truss</p> |
| | Superstructure | | YES | |
| | Substructure | | NO | |
| | Deck | | YES | |
| | Drain | | YES | |
| General | Completeness of design (Y/N) | | | |
| | Kerbs | | YES | |
| | Bearing | | YES | |

| | Parameter | Structure design criteria | Comments |
|-------------------------|---|--|--|
| | Expansion joints Provision for services Standardised/pre-engineered components In situ cast elements Previously used/tested in Australia? Availability of documentation/drawings Room for future expansions | Not required Yes, if required All superstructure NO Yes – recent projects include ToneBridge Bridge Replacement in WA used for Main Roads Western Australia. Blade Bridge for Wollondilly Shire Council. Colongra Power Station NSW – supplied under Ertech. More can be provided if required Technical Brochures, case studies, installation guides, product description etc Permanent structures have the ability to be expanded if necessary, subjected to Engineering approval | line, at each end of the bridge |
| Structural form | Superstructure/deck Abutments Pier Headstock Foundation Wing walls | Steel truss/panel (Bailey) bridge with steel decking Designed by the contractor Not required Designed by the contractor Designed by the contractor Designed by the contractor | The deck is a steel fabricated unit, which is 3.042 m long x 1.050 m wide x 135 mm deep and spans between the top flanges of the bridge transoms |
| Primary material | Superstructure Substructure Bearing Barriers Expansion joints | Galvanised high-strength steel with a polyurethane based anti-skid deck surface N/A Galvanised high-strength steel Galvanised high-strength steel Galvanised high-strength steel | |

| | Parameter | Structure design criteria | Comments |
|---------------------|---|---|--|
| Design | Design standards | AS 5100 (2004), AASHTO | <p>Typical design does not include barriers</p> <p>AS5100 recommends a higher fatigue load than AASHTO</p> |
| | Design traffic load | AS 5100 (2004)/SM1600 | |
| | Other design forces | Earthquake | |
| | | Submergence | |
| | | Scour | |
| | | Barrier impact | |
| | Fatigue | 100 000 cycles of stress due to AASHTO HS25-44 truck or lane loading applied to both lanes of the bridge simultaneously | |
| | Design life | 100 years design life | |
| | Design AADT | Not specified | |
| | Design speed | Not specified | |
| Compliance | Other design assumptions | N/A | <p>Clause 3.1 – DCBOS</p> <p>Clause 4.9.1.1 – DCBOS</p> |
| | Elements not covered in standard design | Substructure | |
| | Compliance to <i>Design Criteria for Bridges and Other Structures</i> | Generally, Yes | |
| | Compliance to TMR specifications | Generally, Yes | |
| Geometry | Departures from TMR specifications | <p>1. Narrow road way</p> <p>2. Trusses are not designed to act as a traffic barrier, but can be attached if required</p> | <p>Clause 3.1 – DCBOS</p> <p>Clause 4.9.1.1 – DCBOS</p> |
| | Typical widths | 3.15 m, 4.20 m, and 7.35 m | |
| | Typical span length | 15.24 m (50 ft) to 60.96 m (200 ft) in modular increments of 3.048 m (10 ft) | |
| | Number of spans | Multiple spans available if needed | |
| | Number of traffic lanes | Dual lane traffic | |
| Site considerations | Pedestrian/cycle lanes | These are available as a Cantilever walkway – 1.5 m wide and can be located on either side or both sides of the bridge | |
| | Adaptability for various site conditions | <p>Skew</p> <p>Grade</p> <p>Horizontal curve</p> <p>Vertical curve</p> | |
| | Required site investigations | Not required for superstructure | |
| | Resistance for corrosive environments (design exposure condition) | All steel components on all Mabey's bridge products come galvanised as standard. Additional coating/protection properties would be considered on a case-by-case basis | |
| | | | |

| | Parameter | Structure design criteria | Comments |
|------------------|--|---|---|
| Constructability | <p>In-line construction</p> <p>Shut down option (full shut/lane shut)</p> <p>Deployment lead time</p> <p>Availability of prefabricated/precast items</p> <p>Special machinery</p> <p>Special skills/techniques required at site</p> <p>Transportability of precast components</p> <p>Weight of heaviest part</p> <p>Size of largest part</p> <p>Time to complete</p> | <p>YES</p> <p>Quick assembly methodology for rapid bridge build and time/cost efficiencies on site</p> <p>If current stock available 1 week, otherwise lead time is dependent on the project</p> <p>Items are usually held in stock in locations around Australia, dependant on the project and the availability of parts – these could be ex stock</p> <p>Crane or launching equipment</p> <p>No special skills needed</p> <p>Stock is transported on semi-trailers from Mabey Depot, all stock is flat packed</p> <p>Dependant on the size of the bridge</p> <p>Dependant on the size of the bridge</p> <p>Quick assembly typical for a 5-bay bridge (15 m) to be built in 2 working days with unskilled crews and minimal plant hire</p> | Import from UK |
| Cost | <p>Approximate Construction cost Per sqm (DECK ONLY)</p> <p>Approximate Annual Maintenance cost</p> | <p>15 m span – \$1800</p> <p>Not specified</p> | Supply cost only. Need to add erecting cost. Hire option is also available |
| Maintenance | <p>Recommended inspection frequency</p> <p>Recommended routine maintenance</p> <p>Durability</p> | <p>Initial visual inspection to be carried out one month after bridge has opened to traffic. All bridges to be inspected at least once every 12 months and as soon as possible after a known flood event</p> <p>All bridges to be inspected at least once every 12 months and as soon as possible after a known flood event</p> <p>Galvanized high strength steel is guaranteed for life of asset – expected to be 100 years in applications of low traffic volume and dry climate conditions</p> | Extra protective coatings can be considered depending on the requirement, budget etc. |
| Considerations | | <p>1.Fatigue design is done for AASHTO design guidelines which uses lower fatigue load than the AS 5100</p> <p>2.Trusses are the main load bearing elements. They are vulnerable to impact but not designed to act as a traffic barrier</p> | |
| References | Mabey website | https://www.mabey.com/au/products/bridging/mabey-compact-200 | |

B.7 Waeger Precast Bridges

Table B 7: Waeger bridge system data

| | Parameter | | Structure design criteria | Comments |
|---------|--|------------------------------|--|--|
| System | Developed by | Waeger Constructions Pty Ltd | | Client can construct themselves if they wish |
| | Supplied by | Waeger Constructions Pty Ltd | | |
| | Construction | Waeger Constructions Pty Ltd | | |
| | Key features | | <ol style="list-style-type: none"> 1. Designs custom to each site location to ensure best fit bridge for each application 2. Adaptable in width and length 3. Multiple or single span construction 4. Simple precast solutions with minimal in situ concrete works | |
| | Advantages | | <ol style="list-style-type: none"> 1. Simple precast solutions with minimal in situ concrete works 2. Fast construction time on site | |
| | Limitations/weaknesses | | <ol style="list-style-type: none"> 1. Maximum 15 m span 2. 15 m decks are 26 t and over 3. Typically bespoke with some cast in situ elements such as pier columns | |
| General | Pre-engineered and standardized? (Y/N) | | To some extent, each bridge is designed specifically for the loading and site conditions for each specific application | |
| | Superstructure | | YES | Precast Waeger deck units |
| | Substructure | | YES | Typically, precast abutment and pier headstock beams. Precast sheeting panels for walled abutments and wing wall as required |
| | Barriers | | YES | Low-protection level Thrie beam or custom concrete/steel barrier combination for higher protection level |
| | Completeness of design (Y/N) | | | Scuppers can be incorporated if required. |
| | Drain | | YES | Typically, longitudinal and cross-fall is sufficient for drainage purposes |
| | Kerbs | | YES | Precast kerb, cast-in situ kerbs. Can accommodate castellated or continuous kerbs |
| | Bearing | | YES | Typical laminated elastomeric bearing pads |

| | Parameter | Structure design criteria | Comments |
|-------------------------|--|---|---|
| | Expansion joints | YES | Joint sealant at transverse joints |
| | Provision for services | YES | Can be accommodated between beams or on outer edge of beams |
| | Standardised/pre-engineered components | To some extent, each bridge is designed specifically for the loading and site conditions for each specific application | |
| | In situ cast elements | Piling/foundations depending on site conditions. Columns for piers and abutments are typically cast in situ (Ø600 round columns) as required | |
| | Previously used/tested in Australia? | Mow Creek bridge – Binnaway, 5 Bridges in Walcha Council etc | Extensively used throughout NSW |
| | Availability of documentation | Concept designs for a range of bridge widths and span configurations can be provided | |
| | Room for future expansions | Simple headstock extensions can be completed, plus additional Waeger deck units can be installed to provide additional bridge width as required | |
| Structural form | Deck | Precast Waeger deck (double T unit) | |
| | Abutments | Precast abutments | |
| | Pier | Typically cast-in situ columns with precast headstock beams | |
| | Headstock | Precast unit | |
| | Foundation | Bored cast in piles, driven concrete piles, driven steel piles | Typically, compatible with all types of foundations |
| | Wing walls | Precast panels with bolted connections | |
| | Relieving slab | Can be precast or cast-in situ depending on site preferences | |
| Primary material | Superstructure | Concrete (prestressed) | |
| | Substructure | Concrete | |
| | Bearing | Laminated elastomeric bearing | |
| | Barriers | Steel/concrete | |
| | Expansion joints | Polyurethane joint sealants | |

| | Parameter | Structure design criteria | Comments |
|---------------------|---|---|--|
| Design | Design standards | AS 5100:2017 | Low-protection level Thrie beam or custom concrete/steel barrier combination for higher protection level |
| | Design traffic load | T44 or SM1600 loading and HLP320 or HLP400 if required | |
| | Other design forces | Earthquake Submergence Scour Barrier impact | |
| | | Site dependent YES Site dependent No/low/regular options available | |
| | Design life | 100 years | |
| | Design AADT | Not specified | |
| | Design speed | Not specified | |
| | Other design assumptions | Not specified | |
| | Elements not covered in standard design | Each design is customised to the specific site application | |
| Compliance | Compliance to <i>Design Criteria for Bridges and Other Structures</i> | Can be compliant where required | |
| | Compliance to TMR specifications | Can be compliant where required | |
| | Departures from TMR specifications | Typically, none | |
| Geometry | Typical widths | 2.3 to 3.5 m per Waeger deck unit. Bridge widths from 4.2 m and up | Can be simply raised from road level with kerb type barrier separation, or other separation barrier as required by the site/client |
| | Typical span length | 6 m to 15 m | |
| | Number of spans | Single or multiple | |
| | Number of traffic lanes | Single or multiple | |
| | Pedestrian/cycle lanes | Can be incorporated easily with additional deck width | |
| Site considerations | Adaptability for various site conditions | Skew Grade Horizontal curve Vertical curve | |
| | | Up to 45° Longitudinal and transvers grades easily accommodated. Single or dual cross-fall Easily accommodated with additional within short bridges, or staggered skew arrangement for long bridges Can be incorporated into substructure elements with little additional work | |
| | Required site investigations | Geotechnical, detailed survey, service locations | |
| | Resistance for corrosive environments (design exposure condition) | All concrete elements can be designed to various exposure classifications in accordance with AS 5100 and AS3600 | |
| | | | |
| | | | |

| | Parameter | Structure design criteria | Comments |
|------------------|---|--|---|
| Constructability | In-line construction | YES | Depends on road alignment and construction of existing bridge. Superstructure system is easily split for staged construction. Substructure can also be designed for staged construction if required |
| | Shut down option (full shut/lane shut) | Either can be accommodated depending on existing bridge construction and road alignments | |
| | Deployment lead time | Typically, 8 to 10 weeks for design development | |
| | Availability of prefabricated/precast items | Typically, 3 to 6 weeks from issue of design drawings | |
| | Special machinery | Mobile cranes for lifting and installation, plus piling equipment to suit site conditions | |
| | Special skills/techniques required at site | Experience in installation of precast/prefabricated items is recommended | |
| | Transportability of precast components | Typically, on flatbed semi-trailers. Larger/heavier deck units may be on extendable semi-trailers or extendable low loaders or jinkers | |
| | Weight of heaviest part | 15 m decks are 26 t and over | |
| Cost | Size of largest part | 15 m long x 2.5 m wide x 0.7 m high | Width can be increased to suit desired bridge width/arrangement |
| | Time to complete deck | Typically, trafficable 2 to 3 days after installation of precast deck | Time to allow for grouting of shear key joints and joint sealants. Products used for this will determine actual trafficable time required |
| | Approximate construction cost per sqrm | Typically, \$1250 to \$2,000/m ² (ex GST) | Includes design of entire bridge, plus supply of abutment/pier headstocks, bridge deck and traffic barriers. Based on single span, dual lane bridges |
| | Approximate annual maintenance cost | Not specified | Designed for minimal maintenance. Typically, replacement of bearings at 40 to 60 years design life. General maintenance on steel components (if used) |
| Maintenance | Recommended inspection frequency | Typical bi-annual inspections recommended | |
| | Recommended routine maintenance | Cleaning of headstock sills. Checking of bolts/connections | |
| | Durability | 100 years design life | |
| Considerations | | | |

| | Parameter | Structure design criteria | Comments |
|------------|----------------|---|----------|
| References | Waeger website | http://www.waeger.com.au/ | |